



Consulting Engineers & Scientists

October 15, 2008

Red Point Development  
8710 North Thornydale Road  
Tucson, Arizona 85742

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Tucson, Arizona 85719  
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Fax 520.792.2539  
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Attn: Mr. Larry Kreis  
General Manager

**RE: Supplemental Recommendations to Geotechnical Engineering Report  
Phase I Residential Development at the Pines  
At the Pines Golf Course  
North of Cortaro Road and West of Interstate 10  
Marana, Arizona  
Terracon Project No. 63045225, Addendum 10, Revision 1**

Terracon has completed the geotechnical engineering report for the Residential Development at the Pines (Terracon Project 63045225, dated December 8, 2004). Between June 2005 and July 2008, nine addendums to the geotechnical report were provided. Two of these addendums addressed pavement recommendations for Continental Links Drive. The remainder primarily addressed a portion of the property where fills between 7 and 27 feet deep exist. Alternative recommendations to mitigate problems arising from potential settlement of this fill were provided.

These recommendations applied to the lots in the area of deep existing fills. These lots included:

Lot 1	Lot 16	Lot 73
Lot 2	Lot 17	Lot 74
Lot 3	Lot 18	Lot 75
Lot 4	Lot 19	Lot 76
Lot 5	Lot 70	Lot 77
Lot 6	Lot 71	Lot 78
Lot 15	Lot 72	Lot 115

We have been requested by Red Point Development to evaluate the possibility of overexcavating and recompacting the loose fill soils in these areas, to depths we believe sufficient to reduce potential structural settlements to levels where an economical foundation design may be feasible.

The deepest fills estimated to be between 17 and 27 feet are expected at Lots 71 to 77. In the areas of Lots 1 to 4, 17 to 19, 70, 78, and 115, we anticipate the depth of fill to be between 10 and 17 feet. In the areas of Lots 5 and 6, 15 and 16 we expect depth of fill between 5 and 10 feet.

The following table shows the depth of overexcavation and recompaction of existing fills we recommend for these areas.

<b>Area</b>	<b>Depth From Existing Grade (ft)</b>
Lots 1 - 4	10
Lots 5 and 6	5
Lot 70	10
Lots 71 - 74	15
Lot 78	10
Lots 75 - 77	15
Lots 15 and 16	5
Lots 17 - 19	10

The attached Figures 1 through 6 show these areas and cross sections of our recommended overexcavation zones. Soils overexcavated and recompacted as engineered fill should be placed in loose lifts not exceeding 12-inches in height, and should be compacted to a minimum of 95 percent of Standard Proctor Density (ASTM D698) near optimum moisture content.

The cross sections indicate access ramps down into each of the excavations with inclinations of about 3.5:1 (horizontal:vertical). These slopes would not need to extend across the entire side of a given excavation, but rather be wide enough to accommodate the excavation equipment. The remainder of that side of the excavation may have slopes as steep as 1:1 (horizontal:vertical). Also, we understand some of the ramps as shown on our cross section may need to be relocated to other portions of the excavations due to existing site walls.

We understand that additional fill will be required due to compaction shrinkage factors and the desire to elevate the finished grades of the lots above their current elevations. All fill imported to the site should meet the requirements for imported fill provided in the original geotechnical report, and should be compacted according the recommendations in the original report.

If these earthwork preparations are completed as recommended, the following criteria for post-tensioned slab-on-ground foundations may be used for design.

**Post-Tensioned Slab Foundation Systems (Compressible Soil Case):** Post-tensioned slab construction can be considered as an alternate foundation system for the project. Post-tensioned slabs should be designed using criteria outlined by the Post-Tensioning Institute<sup>1</sup> for the compressible soil case based on the following:

- Maximum Allowable Bearing Pressure (at surface)..... 1250 psf
- Soil Modulus of Elasticity,  $E_s$  ..... 3000 psi
- Soil Modulus of Subgrade Reaction,  $k$  ..... 200 pci
- Total Soil Movement,  $\delta$  ..... 1 inches\*
- Slab-Subgrade friction coefficient,  $\mu$ 
  - on polyethylene sheeting..... 0.75
  - on cohesionless soils ..... 1.00
  - on cohesive soils..... 2.00

\*Estimated settlement based upon total structure load expressed as a uniform 1250 psf pressure acting over the entire slab area. For structure loads resulting in a pressure less than 1250 psf, a reduced  $\delta$  value could generally be estimated by linear interpolation.

Post-tensioned slabs, thickened or turn-down edges and/or interior beams should be designed and constructed in accordance with the requirements of the Post-Tensioning Institute and the American Concrete Institute. Perimeters of the post-tensioned slabs should bear a minimum of 12 inches below adjacent grades. Subgrades supporting a post-tensioned slab should be prepared as recommended in this report.

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<sup>1</sup>PTI Slab-on-Ground Committee, (2004), *Design and Construction of Post-Tensioned Slabs-on-Ground*, Post-Tensioning Institute, Third Edition.

**Phase I Residential Development at the Pines  
Pines Golf Course  
Terracon Project No. 63045225, Addendum 10, Revision No. 1**

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If you have any questions regarding this letter please contact us.

Sincerely,

**TERRACON**



**Expires 03/31/2009**

Oleg B. Lysyj, P.E.  
Principal

Copies:        Addressee (2)

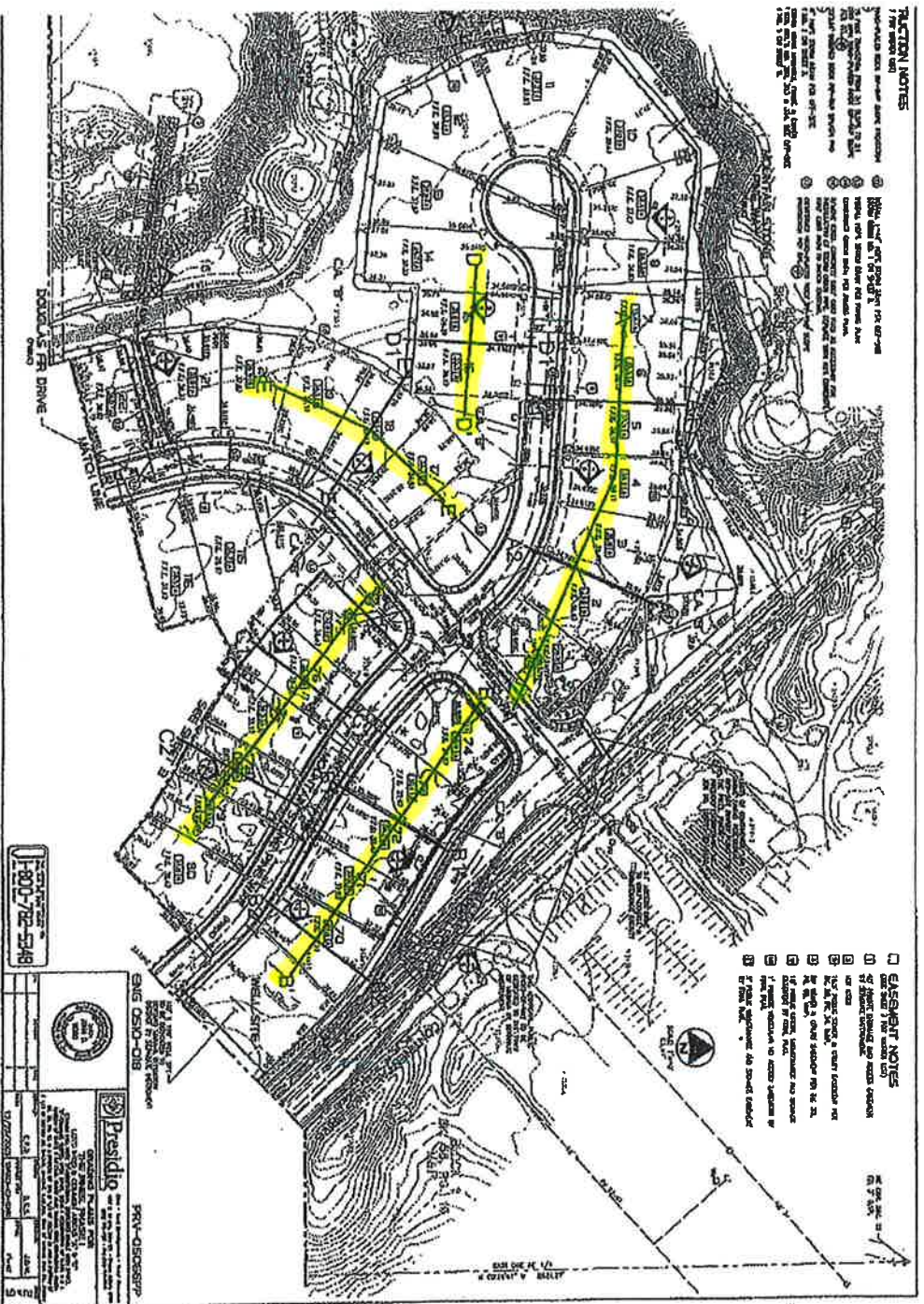
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**GENERAL NOTES**

1. THE EXCAVATION SHALL BE MADE TO THE DEPTH INDICATED ON THE DRAWING. THE EXCAVATION SHALL BE MADE TO THE DEPTH INDICATED ON THE DRAWING. THE EXCAVATION SHALL BE MADE TO THE DEPTH INDICATED ON THE DRAWING.

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  - 10. THE EXCAVATION SHALL BE MADE TO THE DEPTH INDICATED ON THE DRAWING.

100-762-5148

**Presidio**

GENERAL CONTRACTOR

100-762-5148

Project No.	100-762-5148	Scale	AS SHOWN
Contract No.	100-762-5148	Drawn By	JAN
Check No.	100-762-5148	Checked By	JAN
Issue No.	100-762-5148	Issue Date	10/18/03

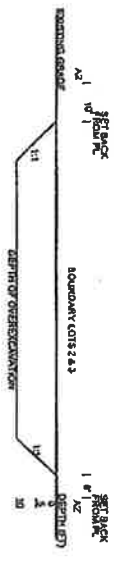
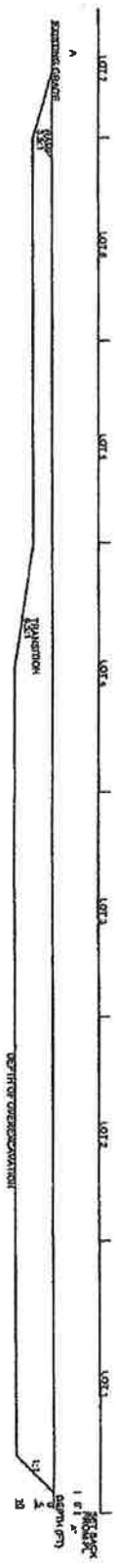
**Terracon**  
Consulting Engineers and Architects

100-762-5148

**FIG. No. 1**

**SITE PLAN OVER EXCAVATION CROSS SECTION**

PHASE 1 LOT 100 UNIT

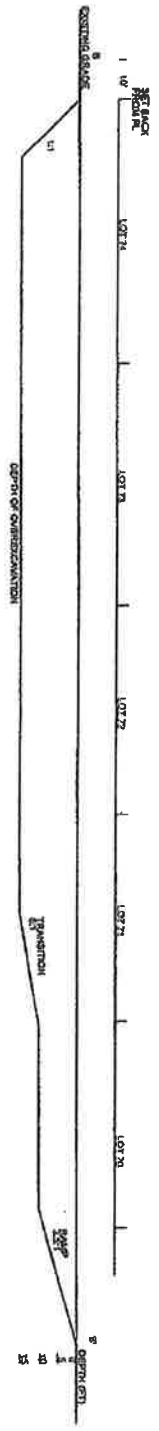


Project Name	001	Project No.	0000002
Client	JNI	Scale	1"=20'
Contract	001	File No.	0000002
Approved	001	Date	10-06-03


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 2111 COLLETT ST  
 VANCOUVER BC V6J 1A6  
 TEL: 604-271-2222

**OVER EXCAVATION CROSS SECTION**  
 THE PHASE 1  
 A SECTIONS

FIG. No.  
**2**



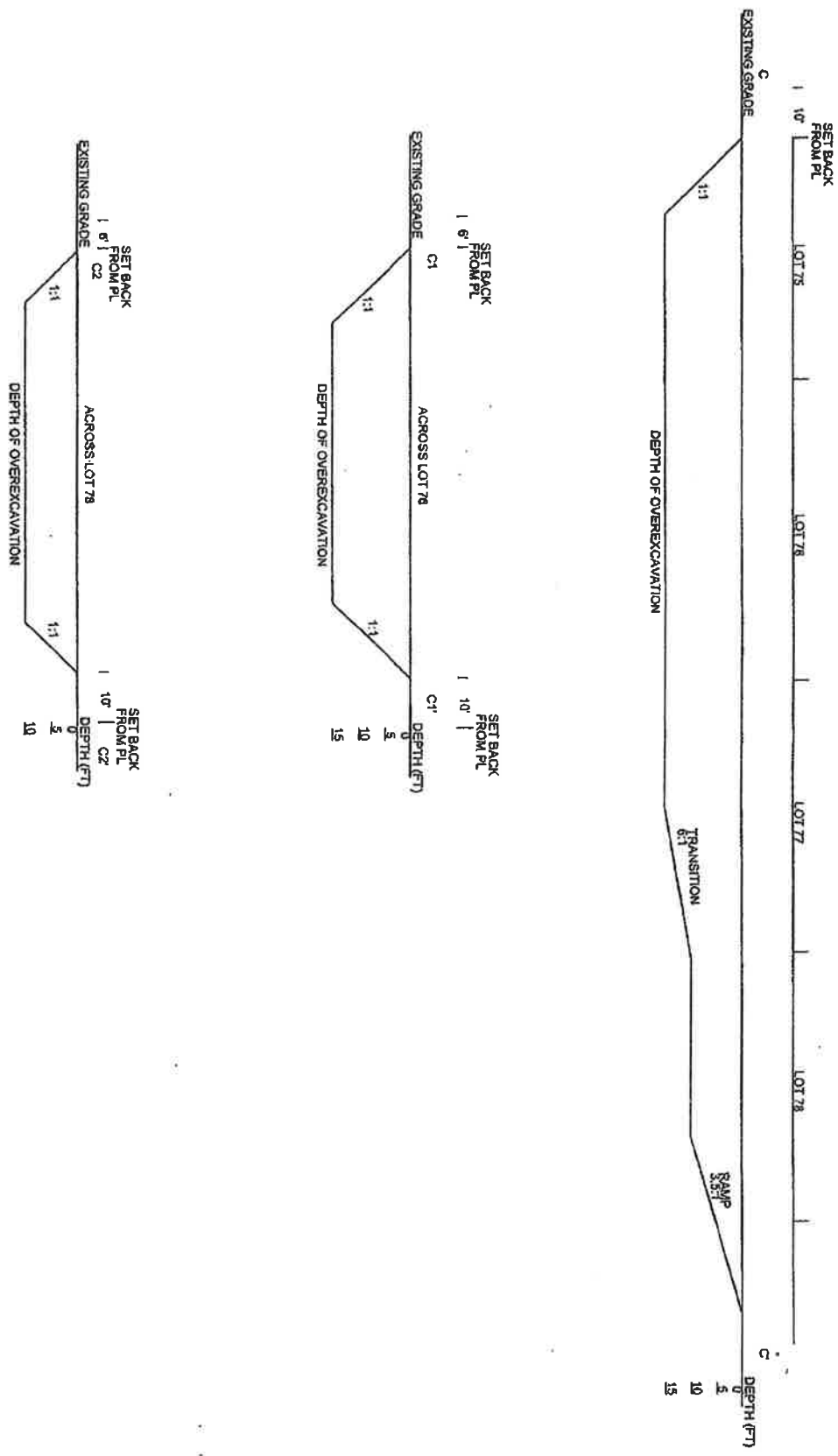
Project No.	001	Project Name	EMERGES
Client	AM	Date	1-28
Contract	001	Project No.	EMERGES
Approval	001	Revision	10-02-01



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 28 ALABAMA AVE. SE  
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**SITE PLAN**  
**OVER EXCAVATION CROSS SECTIONS**  
 THE PRESS PHASE I  
 8 SECTIONS

FIG. No.  
**3**



Project No.	0200322	Sheet No.	122
Drawn By	JMK	Scale	
Checked By	CSK	Date	03/01/22
Approved By	CSK	Drawn By	JMK
DATE	03/01/22	DATE	03/01/22

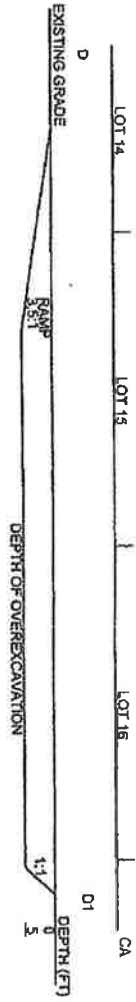
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 200 E. REXFORD, SUITE 600  
 TUCSON, ARIZONA 85701-4008

**SITE PLAN  
 OVER EXCAVATION CROSS SECTIONS  
 THE PRESS HOUSE I  
 C SECTIONS**

ARIZONA

FIG. NO.  
**4**



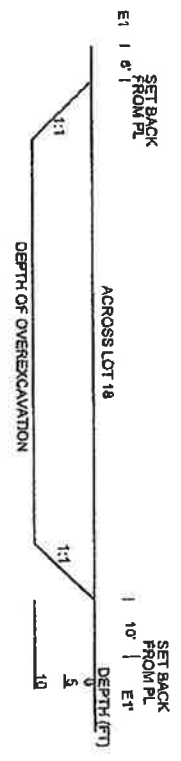
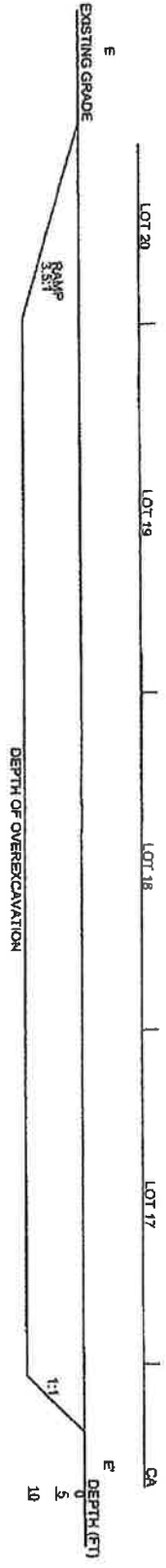


Project No.	0001225
Client	JNH
Contract No.	1-27
Drawn by	CSL
Checked by	CSL
Date	10-06-01

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 2222 WEST 10TH AVENUE  
 SUITE 100  
 DENVER, CO 80202  
 TEL: 303.733.8800  
 FAX: 303.733.8801

THURSDAY  
 OVER EXCAVATION CROSS SECTIONS  
 THE PINE, PAPER 1  
 0 SECTION  
 ARIZONA

FIG. No.  
**5**



Project No.	6003225
Client	7-23
Contract	6003225
Approved	10-05-03

**Terracon**  
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 Tucson, Arizona 85702  
 (520) 297-2500

SITE PLAN  
 OVER EXCAVATION CROSS SECTIONS  
 THE PHASE 1/PHASE 1  
 E SECTION  
 ARIZONA

FIG. No.  
**6**