

**Town of Marana
Impact Fee Study**

**Wastewater Facilities
Infrastructure Improvements Plan**

**Public Report
FINAL**

As approved by the Marana Town Council, September 20, 2022

Prepared by



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September 20, 2022



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1 Introduction

As of July 2021, the Town of Marana Water Department (Marana Water) owns and operates a sewer collection system that currently serves approximately 5,200 sewer connections and conveys approximately 695,000 gallons per day (gpd) of sewage to the Marana Water Reclamation Facility (WRF). The Town of Marana (Town) is expected to grow rapidly in the upcoming years with an anticipated additional 10,362 new sewer connections in the next 10 years.

Arizona Revised Statutes (ARS) §9-463.05 require that Development Impact Fees be based on adopted land use assumptions and an adopted Infrastructure Improvements Plan (IIP). To support the Town in meeting these requirements, Marana Water contracted with HDR to update the Town's IIP which details the Town's existing sewer collection and treatment infrastructure, available capacity, units of demand, and planning for new infrastructure required to serve future proposed development.

The purpose of this IIP update is to summarize the capital improvements required for the Town to meet the demands associated with the next 10 years of growth and to provide the estimated costs of the improvements.

2 Existing Sewer Collection and Treatment Facilities

The Marana collection system has approximately 84.3 miles of gravity mains ranging in size from 8-inch to 24-inch diameter. In addition to gravity pipe, the collection system also has approximately 1,729 manholes and 1.7 miles of 8-inch diameter force main.

Marana Water operates multiple lift stations including the Rillito Vista Lift Station (LS) and the Saguaro Bloom LS. The Rillito Vista LS serves the Rillito Vista development and the Saguaro Bloom LS serves the active service area south of the Santa Cruz River. Marana Water also recently purchased the Adonis Sewer System and has constructed a new lift station and forcemain to provide service for this area. There are 153 active sewer connections in Adonis. The average daily flow is approximately 18,000 gallons per day (gpd).

Marana Water operates the Marana Water Reclamation Facility (WRF) which has a current treatment capacity of 1.5 million gallons per day (MGD) and treats the wastewater collected within the Marana sewer system service area. Through expansion the existing Marana WRF can provide a future treatment capacity of up to 4.5 MGD.

Marana Water also operates the Airport Sewer System which is a small standalone system consisting of approximately 5,000 linear feet (LF) of collection lines, 12 manholes and a septic tank / leach field disposal system rated for a treatment capacity of 19,000 gpd. There are currently no active connections to the Airport Sewer System.

3 Designated Management Area and Active Sewer Service Area

Marana Water is the Designated Management Agency providing wastewater service for the Designated Management Area (DMA) which mainly consists of the western portion of the Town and areas beyond the Town limits to the west and south. The sewer DMA which is served by the existing Marana WRF includes only a portion of the Town's population. Many residential and commercial developments within the Town are located outside the DMA.

Within Marana Water's DMA, the current active sewer service area is generally located along the I-10 corridor north of the Santa Cruz River except for the Saguaro Bloom area which is located south of the Santa Cruz River.

The eastern portion of the Town is outside the Marana DMA and is served by Pima County Regional Wastewater Reclamation Department (PCRWRD) facilities.

The Town and Marana Water's DMA limits are shown in Figure 3-1. Figure 3-2 shows the current active sewer service area.

Figure 3-1. Town and Marana Water's DMA Limits

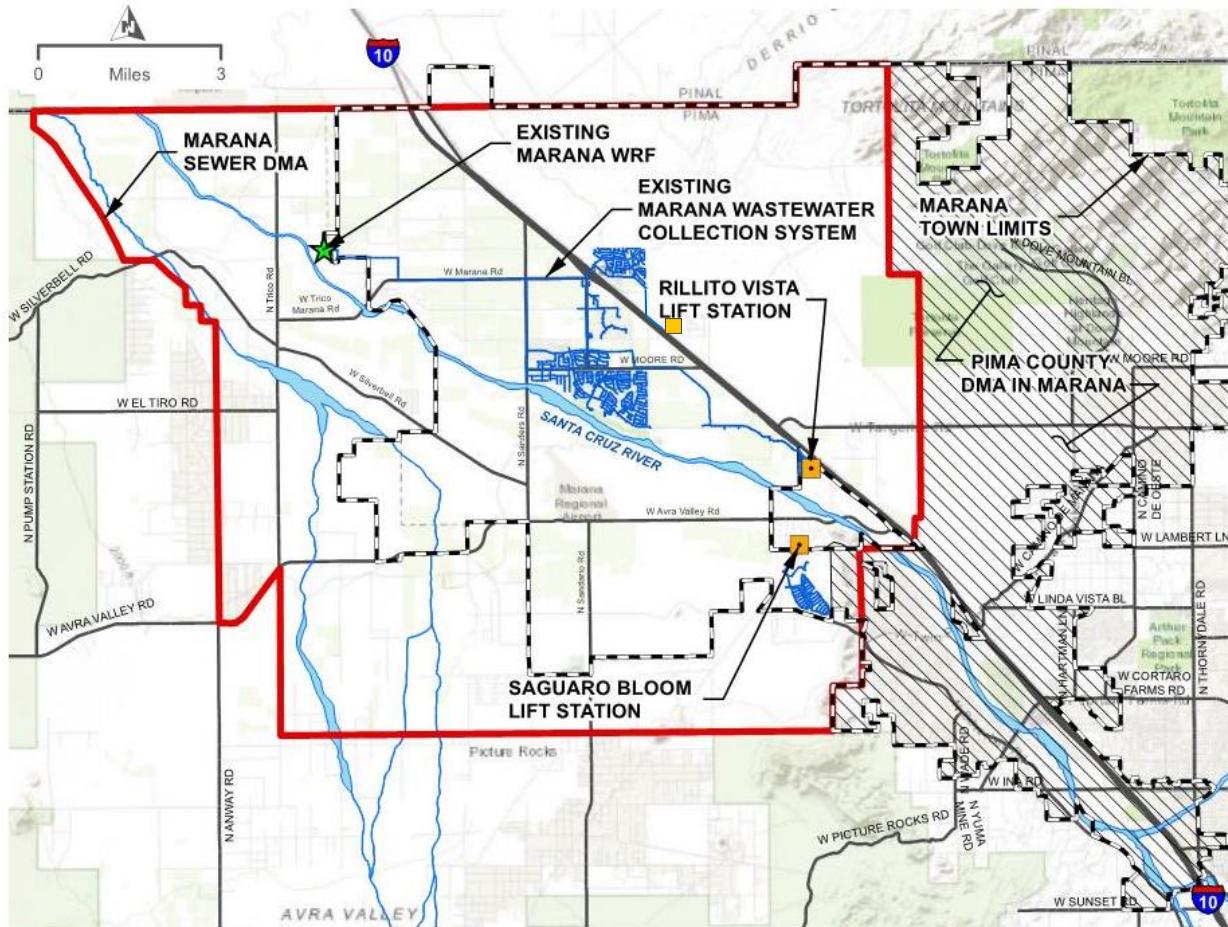
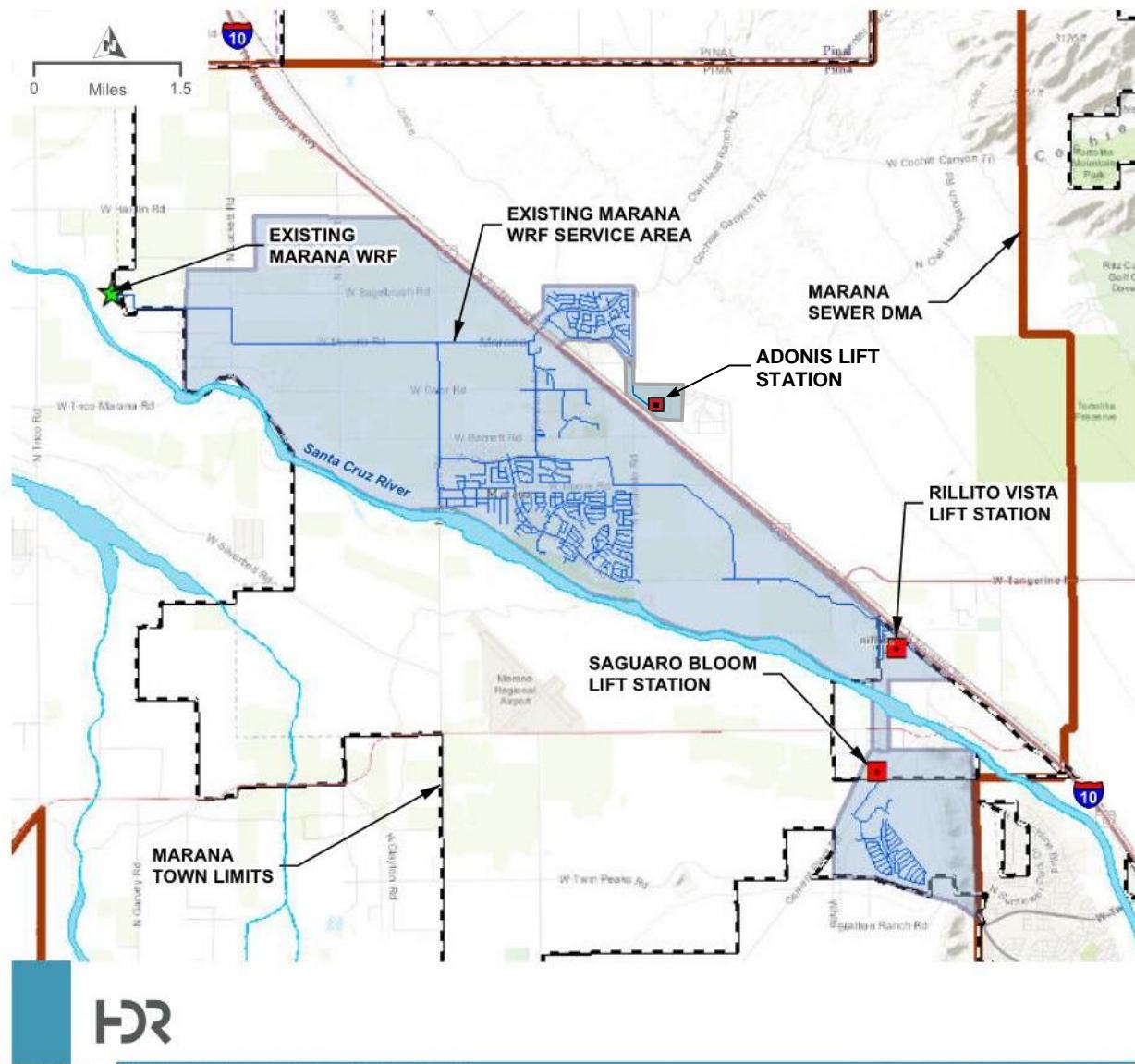


Figure 3-2. Active Sewer Service Area



4 Level of Service

The Town provides sewer collection and treatment for customers within the active sewer service area. The level of service can be defined by the design criteria established by the Town for these systems.

The sewer mains in the collection system are designed to convey the peak wet-weather flow (PWWF). Sewer mains are considered to have adequate capacity if they're able to convey the PWWF with the pipes flowing at depth to diameter (d/D) ratio of 75% or less and without any surcharging.

The Marana WRF is designed to treat the average dry-weather flow (ADWF) for the system and the facility is designed to produce Class A+ reclaimed water as defined by the Arizona Department of Environmental Quality (ADEQ). The treated effluent can be reused, recharged, or discharged to a tributary of the Santa Cruz River.

The level of service provided is the same for both existing and future customers within the active sewer service area.

5 Projected 10-Year Land Use

HDR completed the Sewer Conveyance Master Plan in June 2020 based on future development projections available at that time. Following the completion of the Sewer Conveyance Master Plan the Town has continued to update the projections for the future growth areas, so a revised summary of the commercial and residential developments within the DMA for the 10-year planning horizon associated with this IIP update was provided. The future growth areas for the Town are shown in Figure 5-1, and the estimated additional DUs to be added to the active sewer service area through Year 2032 are summarized in Table 5-1.

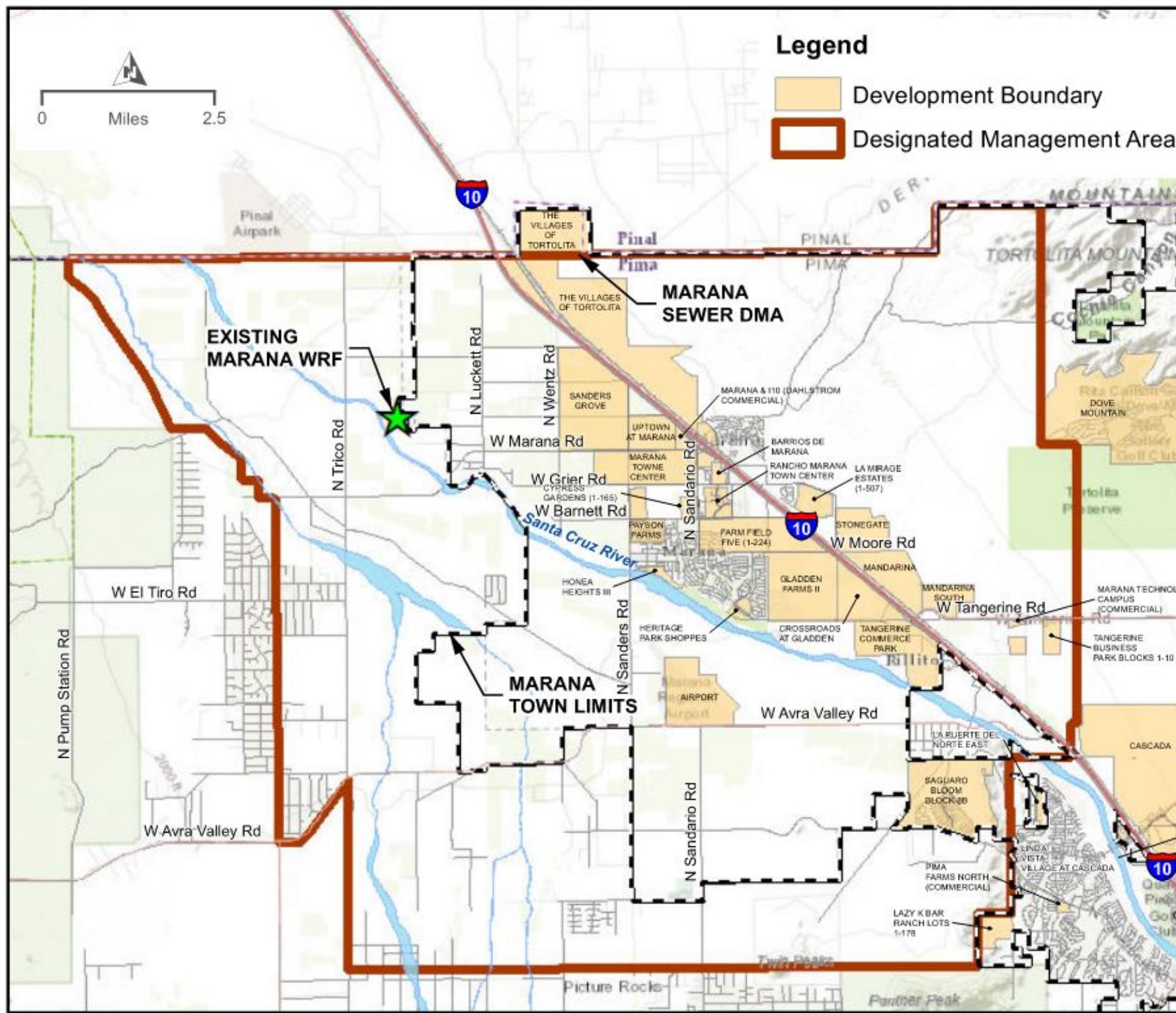
Table 5-1. Future Developments and Estimated DU's

| Development | Buildout DUs ^{a,b} | Existing DUs | Projected DUs from Jul 2021 – Nov 2022 | Additional DUs to be Assigned | Year 2023 – 2027 | Year 2028 – 2032 | Year 2033+ |
|------------------------------|-----------------------------|--------------|--|-------------------------------|------------------|------------------|---------------|
| Mandarina | 2,544 | - | - | 2,544 | 532 | 810 | 1,202 |
| Mandarina South | 830 | - | - | 830 | 40 | 150 | 640 |
| Stonegate | 3,138 | - | - | 3,138 | 300 | 700 | 2,138 |
| Villages of Tortolita | 6,500 | - | - | 6,500 | - | 300 | 6,200 |
| Subtotal East of I-10 | 13,012 | 0 | 0 | 13,012 | 872 | 1,960 | 10,180 |
| Crossroads at Gladden | 672 | - | - | 672 | 160 | 328 | 184 |
| Cypress Gardens | 217 | - | - | 217 | 100 | 117 | - |
| Gladden Farms II | 2,539 | 599 | 351 | 1,589 | 1,421 | - | 168 |
| Gladden Farms Commercial | 88 | - | - | 88 | - | - | 88 |
| Heritage Park Shoppes | 72 | - | - | 72 | 16 | 20 | 36 |
| Marana & I-10 | 148 | - | - | 148 | 12 | 100 | 36 |
| Marana Main Street | 80 | - | - | 80 | 16 | 24 | 40 |
| Marana Towne Center | 4,048 | - | - | 4,048 | 40 | 80 | 3,928 |
| Payson Farms | 457 | - | - | 457 | 457 | - | - |
| Rancho Marana – West | 536 | 307 | 59 | 170 | 150 | 4 | 16 |
| Rancho Marana Town Center | 164 | - | - | 164 | - | 20 | 144 |
| Saguaro Bloom | 2,509 | 1,245 | 564 | 700 | 700 | - | - |
| Sanders Grove | 2,500 | - | - | 2,500 | 100 | 600 | 1,800 |
| Tangerine Commerce Park | 188 | - | - | 188 | 60 | 60 | 68 |
| Uptown at Marana | 930 | - | - | 930 | - | 300 | 630 |
| Vanderbilt Farms | 1,945 | 224 | - | 1,721 | 500 | 1,000 | 221 |
| Villages of Barnett | 251 | - | 81 | 170 | 50 | - | 120 |
| Subtotal West of I-10 | 17,344 | 2,375 | 1,055 | 13,914 | 3,782 | 2,653 | 7,479 |
| Airport | 200 | - | - | 200 | 20 | 20 | 160 |
| Subtotal Airport | 200 | 0 | 0 | 200 | 20 | 20 | 160 |
| Total | 30,556 | 2,375 | 1,055 | 27,126 | 4,674 | 4,633 | 17,819 |

^a Buildout is only within the service areas of the existing WRF and proposed WRF east of I-10

^b Includes commercial developments with assumed density of 4 DUs per acre.

Figure 5-1. Development Activity



6 Existing System Capacity

This section provides an analysis of the existing system capacity as well as the current level of usage and commitments for usage of capacity for the existing wastewater collection and treatment systems.

6.1 Unit Wastewater Flow Rates

The initial step in evaluating the system capacity is establishing the estimated unit wastewater generation rate for the existing sewer collection and treatment system and an equivalency establishing the ratio of a service unit to the various types of land uses (i.e., residential, commercial and industrial, etc.) within the service area.

As part of the Sewer Conveyance Master Plan, HDR developed unit wastewater flow rates for each customer type based on a review of manhole flow monitoring data as well as applicable design and construction criteria contained in the Pima County Regional Wastewater Reclamation Department Engineering Design Standards and the Arizona Administrative Code (AAC).

The following provides a summary of the recommended parameters from the Sewer Conveyance Master Plan.

Gravity Conveyance System:

- Residential unit flow = 216 gpd/DU¹
- Commercial unit flow rate = 1,466 gpd per connection
- PWWF = 499.5 gpd/DU²

Forcemains and Lift Stations:

- Lift stations should be capable of operating at minimum flows and maximum flows:
 - PWWF = 499.5 gpd/DU

Treatment Capacity:

- Average Dry Weather Flow (ADWF)** at 127.6 gal/DU. However, based on the last six months (September 2021 – February 2022) of influent data to the Marana

¹ From Pima County Engineering Design Standards, i.e., 2.7 residents per DU x 80 gallons per capita per day

² Per AAC R18-9-E301(D)(b)(i) for estimated population of 9,500 in sewer collection area, the Peak Dry Weather Peaking Factor is 1.85. This is multiplied against the PWWF peaking factor of 1.25, for a combined peaking factor of 2.31. Note that PWWF peaking factors are not specified in the AAC, Pima County Engineering Design Standards, or in ADEQ Bulletin 11. Recent developer estimates have used a PWWF peaking factor of 25% above PDWF. For the purposes of this IIP, 25% above PDWF was assumed.

WRF and most recent count of residential customer connections this, the ADWF for future forecasting has been revised to 139.6 gal/DU³.

** Note that per ADEQ Bulletin 11 – Minimum Requirements for Design, Submission of Plans and Specifications for Sewage Works and AAC R18-9-B202(A)(9)(a), the design of treatment works should be based on the annual average rate which is based on actual flow rates.

6.2 Existing Treatment System – Required Capacity

A summary of the existing influents flows and available treatment capacity for the Marana WRF is provided in Table 6-1. Influent flows to the Marana WRF have steadily been increasing over the last year, increasing from approximately 550,000 gallons per day (gpd) to over 695,000 gpd as experienced in May 2021 which used as the baseline flow for this report noting that the Town's future development projections are based on a July 2021 starting point.

In anticipation of future growth, the existing Marana WRF recently underwent an expansion from 0.5 MGD to 1.5 MGD. With further expansion the build-out capacity of the Marana WRF is 4.5 MGD.

Based on the existing flows and available treatment capacity, the treatment system has approximately 805,000 gpd of excess capacity to support additional future growth.

Table 6-1. Marana WRF – Existing Wastewater Flows and Available Capacity

| Location | Existing Wastewater Flow (gpd) as of July 2021 |
|--|--|
| East of I-10 | 119,561 ^a |
| West of I-10 | 575,439 ^b |
| Total Existing Wastewater Flow (gpd) | 695,000^c |
| Marana WRF Treatment Capacity (gpd) | 1,500,000 |
| Excess Treatment Capacity Available (gpd) | 805,000 |

^a Estimated based on 784 DUs in San Lucas and 153 DUs in Adonis at ADWF of 127.6 gal/DU

^b Estimated as difference between Marana WRF flows minus flows from east of I-10

^c Based on average influent flows to the Marana WRF in May 2021, as reported by the Town

6.3 Existing Sewer Collection System - Required Capacity

The sewer collection system tributary to the Marana WRF is served by 12 main sewer interceptors that have been labeled A through L. Each interceptor is divided into segments that consist of one or more pipes. For the purposes of evaluating the existing sewer collection system capacity as required for the IIP, the interceptors were broken up into segments based on where wastewater loads are allocated to the system.

The sewer capacity tables are provided in Appendix A. Table A-1 shows the calculated PWWF, the resulting depth to diameter (d/D) ratio for the PWWF, the existing pipe

³ ADWF estimated based on highest average monthly flow (792,000 gal/day in January 2022) and residential customer count of 5,672.

capacity, and the excess pipe capacity available for each interceptor segment. The existing capacity is based on the pipe flowing at 100% full. Excess capacity was determined as the difference between the existing PWWF and existing capacity of the pipe. As shown in the table, all pipes have sufficient capacity to convey the existing wastewater flows and have significant excess capacity available for future growth. However, there are multiple segments in Branches D, J, and K which are at or above 50% capacity and are expected to exceed a d/D ratio above 75% in the coming five years. For the purposes of this analysis, HDR identified pipes with d/D ratios of above 0.70 as potentially requiring capacity improvements. See Appendix B, Figure B-1 for the existing sewer capacity.

7 Projected Capacity Requirements

This section provides a summary of the number of projected service units and the projected wastewater loads necessitated by and attributable to new development in the active sewer service area for the period encompassing the next ten years (i.e., 2023 to 2032). The projected capacity requirements for the 10-year projected growth are also compared with the utilization and available capacity of the existing treatment and sewer collection system infrastructure.

7.1 Treatment Capacity Requirements

Table 7-1 provides a summary of the projected flows from future growth for the period between 2023 to 2027 (5 Year) and 2028 to 2032 (10 Year). The Airport Sewer System has a standalone treatment system and will not be conveyed to the Marana WRF. Table 7-2 provides a summary of the existing wastewater flows, additional flows associated with future growth and a comparison with available treatment capacity at the Marana WRF.

As shown in Table 7-2, the Marana WRF is projected to receive an additional ADWF of 796,977 gpd by the end of 2027 and 1,440,952 gpd by the end of 2032.

- The total expected ADWF at the Marana WRF in November 2027 will be 1.49 MGD as compared to the available capacity of 1.5 MGD, which represents approximately 99% of total capacity.
- The total expected ADWRF at the Marana WRF in November 2032 will be 2.14 MGD as compared to the available capacity of 1.5 MGD, which represents approximately 143% of capacity.

In anticipation of future growth, the existing Marana WRF recently underwent an expansion from 0.5 MGD to 1.5 MGD and with further expansion the build-out capacity of the Marana WRF is 4.5 MGD. The additional flows through Year 2027 take the Marana WRF approximately to its rated capacity, and additional flows through Year 2032 are beyond the Marana WRF current capacity. Design of the expansion of the Marana WRF should begin when ADWF exceeds 75% of capacity, which is expected to occur by early-2025. The expansion phasing for the existing WRF is shown in Figure 7-1, with flows

after 2032 based on projections summarized in Table 6-1 and construction of a new WRF east of I-10 as identified in the 2020 Sewer Master Plan⁴.

Table 7-1. Marana WRF – Future Estimated DU's and Influent Wastewater Flows

| Development | Existing DUs | Projected DUs from Jul 2021 – Nov 2022 | Total DUs to be Assigned 2023 – 2032 | Year 2023 - 2027 | | Year 2028 - 2032 | |
|--------------------------------------|--------------|--|--------------------------------------|------------------|------------------------------------|------------------|------------------------------------|
| | | | | Additional DUs | Additional ADWF (gpd) ^b | Additional DUs | Additional ADWF (gpd) ^b |
| Mandarina | - | - | 1,342 | 532 | 74,267 | 810 | 113,076 |
| Mandarina South | - | - | 190 | 40 | 5,584 | 150 | 20,940 |
| Stonegate | - | - | 1,000 | 300 | 41,880 | 700 | 97,720 |
| Villages of Tortolita | - | - | 300 | - | - | 300 | 41,880 |
| Subtotal East of I-10 | 0 | 0 | 2,832 | 872 | 121,731 | 1,960 | 273,616 |
| Crossroads at Gladden | - | - | 488 | 160 | 22,336 | 328 | 45,789 |
| Cypress Gardens | - | - | 217 | 100 | 13,960 | 117 | 16,333 |
| Gladden Farms II | 599 | 351 | 1,421 | 1,421 | 198,372 | - | - |
| Gladden Farms Commercial | - | - | - | - | - | - | - |
| Heritage Park Shoppes | - | - | 36 | 16 | 2,234 | 20 | 2,792 |
| Marana & I-10 | - | - | 112 | 12 | 1,675 | 100 | 13,960 |
| Marana Main Street | - | - | 40 | 16 | 2,234 | 24 | 3,350 |
| Marana Towne Center | - | - | 120 | 40 | 5,584 | 80 | 11,168 |
| Payson Farms | - | - | 457 | 457 | 63,797 | - | - |
| Rancho Marana – West | 307 | 59 | 154 | 150 | 20,940 | 4 | 558 |
| Rancho Marana Town Center | - | - | 20 | - | - | 20 | 2,792 |
| Saguaro Bloom | 1,245 | 564 | 700 | 700 | 97,720 | - | - |
| Sanders Grove | - | - | 700 | 100 | 13,960 | 600 | 83,760 |
| Tangerine Commerce Park | - | - | 120 | 60 | 8,376 | 60 | 8,376 |
| Uptown at Marana | - | - | 300 | - | - | 300 | 41,880 |
| Vanderbilt Farms | 224 | - | 1,500 | 500 | 69,800 | 1,000 | 139,600 |
| Villages of Barnett | - | 81 | 50 | 50 | 6,980 | - | - |
| Subtotal West of I-10 | 2,375 | 1,055 | 6,435 | 3,782 | 527,967 | 2,653 | 370,359 |
| Total | 2,375 | 1,055 | 9,267 | 4,654 | 649,698 | 4,613 | 643,975 |
| Airport Sewer System ^a | - | - | 40 | 20 | 2,792 | 20 | 2,792 |
| Subtotal Airport Sewer System | 0 | 0 | 40 | 20 | 2,792 | 20 | 2,792 |

^a The Airport Sewer System has a standalone treatment system and will not be conveyed to the Marana WRF.

^b Future ADWF projections based on 139.6 gal/DU

⁴ The 2020 Master Plan recommends flows from Villages of Tortolita, Adonis, and San Lucas be diverted to a new WRF located east of I-10 by 2035, and that by 2050 flows from all developments east of I-10 be diverted to the new WRF. High and low projections reflect a range of future residential housing density between 0.5 to 2 DUs per acre within 3,600 acres located west of I-10.

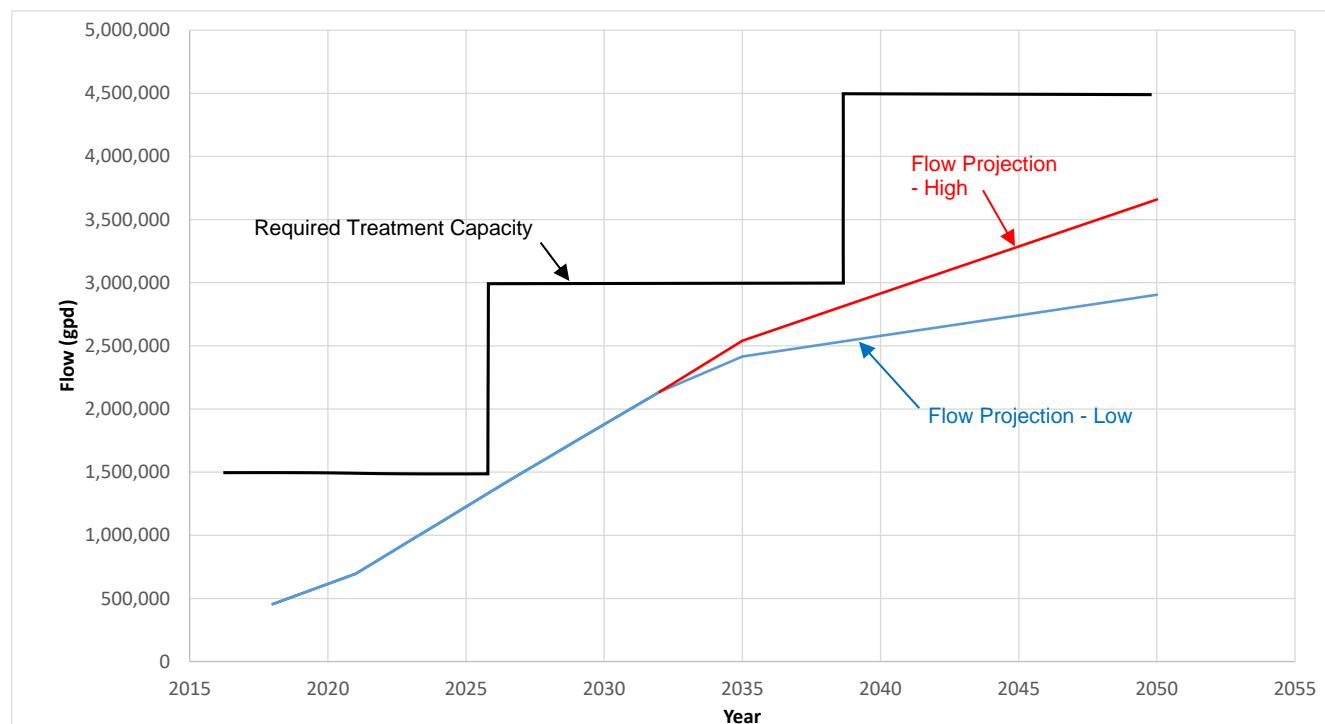
Table 7-2. Marana WRF – Treatment Capacity Summary

| Period | Existing & Projected Wastewater Flow (gpd) | Additional ADWF (gpd) ^b | | Total Wastewater Flow (gpd) | Available Treatment Capacity (gpd) | Excess Treatment Capacity Available (gpd) |
|---------------------|--|------------------------------------|----------------------|-----------------------------|------------------------------------|---|
| | | East of I-10 | West of I-10 | | | |
| Jul 2021 – Nov 2022 | 695,000 | 0 | 147,278 ^a | 842,278 | 1,500,000 | 657,722 |
| 2023-2027 (5 Year) | 842,278 | 121,731 | 527,967 | 1,491,976 | 1,500,000 | 8,024 |
| 2028-2032 (10 Year) | 1,491,976 | 273,616 | 370,359 | 2,135,951 | 1,500,000 | (635,951) |

^a Includes Gladden Farms II – 351 DUs, Rancho Marana West – 59 DUs, Saguaro Bloom – 564 DUs, Villages of Barnett – 81 DUs for a total of 1,055 DUs as shown in Table 7-1. When multiplied by ADWF projection of 139.6 gal/DU, the total projected flow for these developments is 139,042 gpd.

^b ADWF projections based on 139.6 gal/DU

Figure 7-1. Phased Expansion of the Existing WRF



7.2 Sewer Collection System Capacity Requirements

Pipe capacity tables for future scenarios are shown in Appendix A and shows the capacity of the existing sewer collection system to accommodate the growth from 2023 to 2027 if no improvements were made. Appendix A shows the projected flow (i.e., PWWF), the resulting d/D ratio for the PWWF, the existing pipe capacity, and the excess pipe capacity available for each interceptor segment for the 2023 to 2027 and 2028 to 2032 planning periods, respectively.

For the 2023 – 2027 planning period, Appendix A shows that the pipe capacity is 1.00 for portions of Branches I, J, and K, indicating surcharged conditions if no improvements

were made. There are three pipe segments in Branch D that are above 0.70 but are not exacerbated by future growth. Based on these results, 2023 - 2027 improvements were identified (see Section 8.1) to alleviate the surcharging in the identified pipe segments. For the 2023 to 2027 planning period, the proposed gravity sewer improvements result in no pipes experiencing a d/D ratio above 0.70, other than the segments in Branch D which are slightly above 0.70.

For the 2028 to 2032 planning period, Appendix A shows the pipe capacity is 1.00 for portions of Branches B and G, indicating surcharged conditions. There are some pipe segments in Branch B with d/D values between 0.70 and 0.75 but are not exacerbated by future growth and were not susceptible to surcharging, so no future improvements were identified. Based on these results, improvements were identified (see Section 8.2) to alleviate the surcharging in the identified pipe segments. For the 2028 to 2032 planning period, the proposed gravity sewer improvements result in a reduction of d/D ratio from 1.00 to approximately 0.70 – 0.77, with no surcharging occurring anywhere in the system.

Figures showing the location of the pipes with insufficient capacity are included in Appendix B.

8 Capital Improvements

The Town of Marana is expected to develop rapidly over the next 10-15 years, requiring significant investment in infrastructure. HDR has identified several sewer infrastructure capital improvements that will provide Marana Water sufficient conveyance and treatment capacity to accommodate the Town's anticipated growth through the Year 2030 and beyond. The capital improvements required to support the Town's anticipated future growth are described in the following sections.

8.1 2023 – 2027 (5 Year) Infrastructure Improvements

Based on the results of model analysis completed for the Sewer Conveyance Master Plan it was determined the existing sewer collection system has available capacity to convey current peak wet weather flows while maintaining pipes at less than 75% capacity and without surcharging. However, new developments are anticipated east of I-10 in the 10 Year planning period and there is currently no infrastructure in place to support sewer service to these areas. The infrastructure improvements required for the 2023 to 2027 (5 Year) planning phase include gravity lines, lift stations, and force mains that are associated with new infrastructure needed to support sewer service to these new developments and infrastructure needed to increase the capacity of the existing gravity network west of I-10. The existing WRF has insufficient capacity to accommodate the projected growth for the (5 Year) planning phase and will require expansion.

Adonis

There are currently 153 DUs in Adonis, with sewer flows collected at two sewage lagoons located northwest of the development. Marana Water has constructed a new lift station and forcemain to direct flows from Adonis to the San Lucas gravity system. With the addition of the Adonis flows to the gravity system, there is sufficient capacity

available in the gravity main that connects San Lucas to the collection system west of I-10 to accommodate an additional 600 DUs before it surcharges. Since the lift station and force main have already been constructed, the capital costs associated with Adonis are not included as part of the CIP.

Stonegate, Mandarina, and Mandarina South (Improvement 1) – Complete

The new lift station and forcemain serving Adonis was not sized to receive flows from Stonegate, Mandarina, and Mandarina South. Improvements will be required to support any future growth associated with Mandarina and other southeast developments.

A new 15-inch gravity line was constructed by Mandarina LLC to convey flows across I-10 and connect to the Town's existing gravity network west of I-10. The new 15-inch pipe was installed along West Moore Road and connects to the existing 18-inch north-south gravity sewer line at the intersection of West Moore Road and North Clark Farms Boulevard that connects to the existing gravity line from Saguaro Bloom. It is anticipated the new 15-inch gravity line was sized for ultimate buildout in conjunction with other improvements and will have capacity to serve up to 1,000 DUs associated with Mandarina and other southeast developments.

The existing gravity network west of I-10 has sufficient capacity to accommodate up to 250 DUs from these southeast developments, in addition to the anticipated growth on the west side of I-10, before additional gravity conveyance capacity to the existing WRF is required.

Reducing Flows Along Sandario Road (Improvements 2 and 3)

Improvements 2 and 3 are associated with increasing the capacity of the existing gravity network west of I-10. These improvements will also reduce flows flowing north on Sandario Road and provide flow relief to Branch F (see Figure A-2) which would exceed its maximum design capacity without these improvements.

Approximately 4,600 linear feet of 21-inch gravity lines will be required to augment the existing gravity network. A new 21-inch line will tie into the stubbed out gravity line at Tangerine Farms Road at Clark Farms Road and will continue north to Barnett Road (Improvement 2). The 21-inch line will continue west on Barnett Road to Sanders Road and tie into the existing sewer line on Sanders Road (Improvement 3).

Gravity System Improvements Along Sanders Road and Marana Road (Improvements 4 – 7)

The model results in Table A-1 show that without improvements, future sewer flows along Sanders Road and Marana Road will cause the existing sewer interceptors to surcharge. Approximately 19,300 linear feet of new gravity lines will be required to augment the existing gravity network.

The Town requested HDR evaluate two options for augmenting the sewer line along Sanders Road:

- Option 1: Install a parallel 21-inch sewer line from Barnett Road to Marana Road to augment the existing 15-inch sewer line.

- Option 2: Replace the existing 15-inch sewer line from Barnett Road to Marana Road with a new 27-inch sewer line. This option would be beneficial to the Town in not having to maintain two parallel lines, and also because it would eliminate the need for the Town to rehabilitate the existing manholes on the 15-inch line which the Town projects will cost \$731,282.

HDR evaluated the 21-inch and 27-inch options on Sanders Road, which are presented in Tables 8-1 and 8-2, respectively. As shown on Table 8-2, the 27-inch consolidated line will be lower in cost with the added benefit of fewer manholes and less pipe for the Town to maintain. HDR recommends this as the preferred option.

For augmenting the existing sewer capacity along Marana Road west of Sanders Road, the 2020 Sewer Conveyance Master Plan proposed a 27-inch new gravity line run north to Sagebrush Road, and then turn west, tying into the 24-inch existing gravity line at Luckett Road.

The new 27-inch gravity lines on Sanders Road and Sagebrush Road (Improvements 4, 5, 6 and 7) are expected to be required by 2024. As indicated by the results of the existing capacity evaluation (see Table A-1), Branches K and L will surcharge by the end of the 2023 – 2027 (5 Year) planning phase without improvements being made. The improvements are shown on Figure 8-1.

Marana WRF Expansion (Improvement 8)

If all projected development within the 10 year planning period occurs, the influent to the existing Marana WRF is anticipated to reach 99% of the current capacity of 1.5 MGD by November 2027. As such it's recommended the WRF expansion to 3.0 MGD be completed before then.

The Town's approach is to begin designing and constructing a new phase of expansion for its treatment facilities when the influent flow reaches 75% of the capacity of the facility. Per the current flow projections, by early 2025 the influent flow to the WRF will reach 1.125 MGD which is 75.0% of the WRF capacity, the regulatory point for beginning design.

Construction of the WRF expansion should begin prior to reaching 90% of capacity, which is expected to occur by late 2026. The influent flows are expected to reach 1.5 MGD by late 2027 and, assuming two years for construction, construction of the WRF expansion would need to start in late 2025. HDR estimates a 12 – 18 month design period prior to construction and recommends starting design in mid-2024.

A summary of the required sewer system improvements with estimated construction costs are provided in Table 8-1. The improvements are shown on Figure 8-1 and are designated with an Improvement ID number.

Table 8-1. 2023 – 2027 (5 Year) Capital Improvement Summary, with Parallel 15-inch and 21-inch Lines on Sanders Rd (Improvement 5)

| Improvement ID | Infrastructure Component | | Construction Cost ^a |
|----------------|--------------------------------------|---------------------------------|--------------------------------|
| 1 | Gravity Pipe | 15-inch x 3,334 LF | \$0 ^b |
| 2 | Gravity Pipe | 21-inch x 800 LF | \$325,297 ^c |
| 3 | Gravity Pipe | 21-inch x 3,600 LF | \$2,463,177 |
| 4 | Gravity Pipe | 21-inch ^e x 5,300 LF | \$3,613,417 ^d |
| 5 | Gravity Pipe | 27-inch x 2,800 LF | \$1,696,982 |
| 6 | Gravity Pipe | 27-inch x 5,300 LF | \$3,034,513 |
| 7 | Gravity Pipe | 27-inch x 5,900 LF | \$3,371,261 |
| 8 | Existing WRF Design and Construction | 1.5 MGD | \$39,161,113 |
| TOTAL | | | \$53,665,760 |

^a These are total estimated project costs including direct construction costs, contingency, contractor fees, design fees, and other direct costs. Fully loaded costs for each improvement project are provided in Appendix C.

^b See construction drawings for Town of Marana Public Sewer Improvement Plan Mandarina Offsite Sewer S-2020-007 / ENG2002-003. Project will be funded by developer, so costs are not included in the CIP.

^c Construction cost of \$309,807 provided by Meritage Homes, escalated by 5% per the Town's request.

^d Cost of the 21-inch line does not include the \$731,282 maintenance cost associated with rehabilitating the existing 15-inch line.

^e Flows along Sanders Road will be conveyed by a new 21-inch pipeline and parallel existing 15-inch pipeline.

Table 8-2. 2023 – 2027 (5 Year) Capital Improvement Summary, with Consolidated 27-inch Line on Sanders Road (Improvement 5)

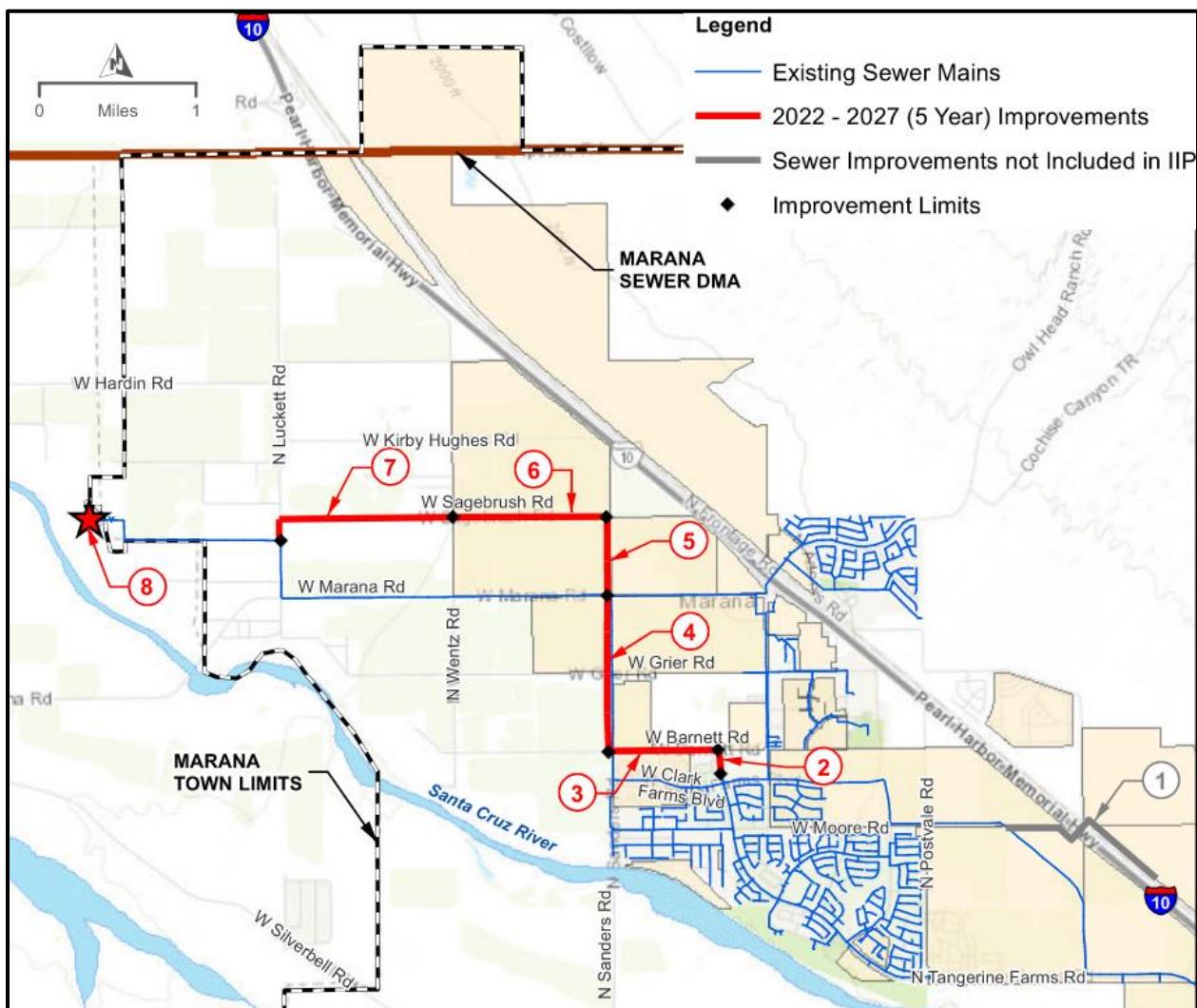
| Improvement ID | Infrastructure Component | | Construction Cost ^a |
|----------------|--------------------------------------|--------------------|--------------------------------|
| 1 | Gravity Pipe | 15-inch x 3,334 LF | \$0 |
| 2 | Gravity Pipe | 21-inch x 800 LF | \$325,297 ^b |
| 3 | Gravity Pipe | 24-inch x 3,600 LF | \$2,745,062 |
| 4 | Gravity Pipe | 27-inch x 5,300 LF | \$3,066,315 ^c |
| 5 | Gravity Pipe | 27-inch x 2,800 LF | \$1,696,982 |
| 6 | Gravity Pipe | 27-inch x 5,300 LF | \$3,034,513 |
| 7 | Gravity Pipe | 27-inch x 5,900 LF | \$3,371,261 |
| 8 | Existing WRF Design and Construction | 1.5 MGD | \$39,161,113 |
| TOTAL | | | \$53,075,246 |

^a These are total estimated project costs including direct construction costs, contingency, contractor fees, design fees, and other direct costs. Fully loaded costs for each improvement project are provided in Appendix C.

^b Construction cost of \$309,807 provided by Meritage Homes, escalated by 5% per the Town's request.

^c The total cost of constructing Improvement 4 is \$4,443,935. The proportional flow from the existing 15-inch line that would be replaced by Improvement 4 represents approximately 31% of the total flow capacity. Therefore, on a capacity basis \$3,066,315 would be assigned to the 27-inch line and \$1,377,620 to the existing 15-inch line.

Figure 8-1. 2023 – 2027 (5 Year) Capital Improvement Projects



8.2 2028 – 2032 (10 Year) Infrastructure Improvements

The infrastructure improvements required for the 2023 to 2027 (5 Year) planning phase include gravity lines, lift stations, and force mains that are associated with new infrastructure needed to support sewer service to these new developments and infrastructure needed to increase the capacity of the existing gravity network west of I-10.

Based on the results of model analysis, new developments located southeast of I-10 (Stonegate, Mandarina, Mandarina South) will result in flows that are in excess of the available capacity, specifically along the 15-inch and 18-inch sewer main that serve Saguaro Bloom. Furthermore, new developments located northeast of I-10 (Villages of Tortolita) will need to be conveyed to the existing sewer lines serving the Adonis and San Lucas development. The combined additional flows from developments located east of I-10 will require a new parallel pipe from the terminus of Improvement 7 to the existing WRF. The expanded WRF (Improvement 8) will have sufficient capacity to accommodate the projected growth for 10 Year planning phase.

New Parallel Gravity Main to Existing WRF (Improvement 9)

The existing 24-inch gravity main feeding the WRF will begin surcharging after developments both east and west of I-10 exceed 10,000 EDUs. This will require adding a new parallel 27-inch gravity line.

New Parallel Gravity Main Along Saguaro Bloom Transmission Line (Improvement 10)

Per the 2020 Master Plan, sewer flows from developments southeast of I-10 (Stonegate, Mandarina, Mandarina South) were to be conveyed to a new WRF located northeast of I-10 after exceeding 1,000 EDUs. The revised development projections show that the southeast developments will exceed 2,500 EDUs by 2032. The existing 15-inch gravity line (see Improvement 1) has sufficient capacity to convey new development flows through 2032, but these flows will result in surcharging along the 15-inch and 18-inch gravity lines downstream of the 15-inch connection point. This will require adding a new parallel 15-inch gravity line.

Villages of Tortolita (Improvement 11)

Initial growth is anticipated in the southern portion of the Villages of Tortolita. Sewer flows from the south will flow by gravity through an 18-inch line northwest towards a lift station and will be pumped back via a 6-inch force main to the existing gravity system serving San Lucas. The maximum size of the lift station is limited to serving approximately 600 EDUs from the Villages of Tortolita before surcharging the San Lucas and Adonis service line under I-10, or approximately 210 gpm pumping capacity. It should be noted the 18-inch gravity line is sized to accommodate ultimate buildout when flows from San Lucas, Adonis, and new developments to the southeast will be directed to the future WRF that will serve all developments on the east side of I-10.

Table 8-3. 2028 – 2032 (10 Year) Capital Improvement Summary

| Improvement ID | Infrastructure Component | Size / Quality | Construction Cost ¹ |
|----------------|--|---|--------------------------------|
| 9 | Gravity Pipe | 27-inch x 5,300 LF | \$3,034,513 |
| 10 | Gravity Pipe | 15-inch x 11,500 LF | \$5,809,844 |
| 11 | Gravity Pipe Lift Station Force Main | 18-inch x 6,800 LF 210 gpm capacity 6-inch x 6,800 LF | \$4,500,690 |
| TOTAL | | | \$13,345,048 |

¹ These are total estimated project costs including direct construction costs, contingency, contractor fees, design fees, and other direct costs. Fully loaded costs for each improvement project are provided in Appendix C.

Figure 8-2. 2028 – 2032 (10 Year) Capital Improvement Projects



Appendix A: Pipe Capacity Tables

Appendix A

Town of Marana Sewer Infrastructure Improvement Plan (IIP)

Sewer Pipe Capacity Tables

| Existing System | | | | | 2022 to 2027 (5 Year) without Improvements | | | | 2022 to 2027 (5 Year) with Improvements | | | | 2027 to 2032 (10 Year) without Improvements | | | | 2027 to 2032 (10 Year) with Improvements | | | | Improvement Notes | | | | | |
|--------------------|---------|----------|---------------|-------------|--|------|-------------------------|---------------------------------|---|------|--------------------------|---------------------------------|---|------|--------------------------|---------------------------------|--|------|--------------------------|---------------------------------|----------------------|------|--------------------------|---------------------------------|-----------------------------------|--|
| Interceptor Branch | Segment | Model ID | Diameter (in) | Length (ft) | Existing Flow (MGD) | d/D | Existing Capacity (MGD) | Excess Capacity Available (MGD) | Projected Flow (MGD) | d/D | Projected Capacity (MGD) | Excess Capacity Available (MGD) | Projected Flow (MGD) | d/D | Projected Capacity (MGD) | Excess Capacity Available (MGD) | Projected Flow (MGD) | d/D | Projected Capacity (MGD) | Excess Capacity Available (MGD) | Projected Flow (MGD) | d/D | Projected Capacity (MGD) | Excess Capacity Available (MGD) | | |
| A | 1 | 193 | 12 | 186 | 0.33 | 0.29 | 1.80 | 1.47 | 0.41 | 0.33 | 1.71 | 1.30 | 0.41 | 0.33 | 1.71 | 1.30 | 0.41 | 0.33 | 1.71 | 1.30 | 0.41 | 0.33 | 1.71 | 1.30 | New Pipe - 2027 to 2032 (10 Year) | |
| A | 2 | 194 | 12 | 296 | 0.33 | 0.30 | 1.63 | 1.30 | 0.41 | 0.34 | 1.63 | 1.22 | 0.41 | 0.34 | 1.63 | 1.22 | 0.41 | 0.34 | 1.63 | 1.02 | 0.60 | 0.42 | 1.63 | 1.02 | | |
| A | 3 | 964 | 12 | 558 | 0.33 | 0.29 | 1.74 | 1.41 | 0.41 | 0.33 | 1.73 | 1.32 | 0.41 | 0.33 | 1.73 | 1.32 | 0.41 | 0.34 | 1.52 | 1.11 | 0.60 | 0.41 | 1.73 | 1.13 | | |
| A | 4 | 85 | 12 | 285 | 0.33 | 0.32 | 1.52 | 1.19 | 0.41 | 0.35 | 1.52 | 1.11 | 0.41 | 0.35 | 1.52 | 1.11 | 0.41 | 0.31 | 1.94 | 1.53 | 0.60 | 0.44 | 1.52 | 0.92 | | |
| A | 5 | 84 | 12 | 64 | 0.33 | 0.31 | 1.58 | 1.25 | 0.41 | 0.31 | 1.94 | 1.53 | 0.41 | 0.35 | 1.56 | 1.16 | 0.41 | 0.35 | 1.56 | 1.16 | 0.60 | 0.38 | 1.94 | 1.34 | | |
| A | 6 | 83 | 12 | 282 | 0.33 | 0.31 | 1.58 | 1.25 | 0.41 | 0.30 | 1.63 | 1.30 | 0.41 | 0.34 | 1.62 | 1.22 | 0.41 | 0.34 | 1.62 | 1.22 | 0.60 | 0.43 | 1.56 | 0.96 | | |
| A | 7 | 82 | 12 | 88 | 0.33 | 0.30 | 1.63 | 1.30 | | | | | | | | | | | | | | | | | | |
| A NEW | 1 | 5080 | 18 | 6,745 | | | | | | | | | | | | | | | | | | | | | | |
| B | 1 | 69296 | 15 | 49 | 0.40 | 0.32 | 1.80 | 1.40 | 0.94 | 0.53 | 1.72 | 0.78 | 0.94 | 0.53 | 1.72 | 0.78 | 0.94 | 0.54 | 1.72 | 0.75 | 0.96 | 0.54 | 1.72 | 0.75 | | |
| B | 2 | 90512 | 12 | 43 | 0.40 | 0.40 | 1.16 | 0.76 | 0.94 | 0.68 | 1.17 | 0.23 | 0.94 | 0.68 | 1.17 | 0.23 | 0.94 | 0.69 | 1.17 | 0.20 | 0.96 | 0.71 | 1.14 | 0.17 | | |
| B | 3 | 90513 | 12 | 205 | 0.40 | 0.41 | 1.13 | 0.73 | 0.94 | 0.69 | 1.14 | 0.20 | 0.94 | 0.70 | 1.12 | 0.19 | 0.94 | 0.69 | 1.14 | 0.20 | 0.96 | 0.71 | 1.12 | 0.16 | | |
| B | 4 | 90514 | 12 | 205 | 0.40 | 0.41 | 1.13 | 0.73 | 0.94 | 0.69 | 1.14 | 0.20 | 0.94 | 0.69 | 1.14 | 0.20 | 0.94 | 0.71 | 1.14 | 0.17 | 0.96 | 0.71 | 1.14 | 0.17 | | |
| B | 5 | 90515 | 12 | 240 | 0.40 | 0.41 | 1.13 | 0.73 | 0.94 | 0.69 | 1.13 | 0.19 | 0.94 | 0.69 | 1.13 | 0.19 | 0.94 | 0.71 | 1.14 | 0.17 | 0.96 | 0.71 | 1.13 | 0.16 | | |
| B | 6 | 90516 | 12 | 410 | 0.40 | 0.41 | 1.13 | 0.73 | 0.94 | 0.69 | 1.15 | 0.21 | 0.94 | 0.69 | 1.15 | 0.21 | 0.94 | 0.70 | 1.15 | 0.18 | 0.96 | 0.70 | 1.15 | 0.18 | | |
| B | 7 | 90517 | 12 | 410 | 0.40 | 0.41 | 1.15 | 0.75 | 0.94 | 0.69 | 1.18 | 0.25 | 0.94 | 0.67 | 1.18 | 0.25 | 0.94 | 0.69 | 1.18 | 0.22 | 0.96 | 0.69 | 1.18 | 0.22 | | |
| B | 8 | 90518 | 12 | 162 | 0.40 | 0.40 | 1.19 | 0.79 | 0.98 | 0.70 | 1.19 | 0.20 | 0.98 | 0.70 | 1.19 | 0.20 | 0.98 | 0.70 | 1.19 | 0.13 | 1.05 | 0.73 | 1.19 | 0.13 | | |
| B | 9 | 90519 | 12 | 163 | 0.40 | 0.40 | 1.19 | 0.79 | 0.98 | 0.70 | 1.19 | 0.20 | 0.98 | 0.70 | 1.19 | 0.20 | 0.98 | 0.70 | 1.19 | 0.13 | 1.05 | 0.73 | 1.19 | 0.13 | | |
| B | 10 | 90520 | 12 | 305 | 0.40 | 0.40 | 1.19 | 0.79 | 0.98 | 0.69 | 1.19 | 0.20 | 0.98 | 0.69 | 1.19 | 0.20 | 0.98 | 0.69 | 1.19 | 0.14 | 1.05 | 0.73 | 1.19 | 0.14 | | |
| B | 11 | 90521 | 12 | 283 | 0.40 | 0.40 | 1.19 | 0.79 | 0.98 | 0.69 | 1.19 | 0.21 | 0.98 | 0.69 | 1.19 | 0.21 | 0.98 | 0.70 | 1.19 | 0.14 | 1.05 | 0.73 | 1.19 | 0.14 | | |
| B | 12 | 90522 | 12 | 343 | 0.40 | 0.40 | 1.18 | 0.78 | 0.98 | 0.70 | 1.18 | 0.20 | 0.98 | 0.70 | 1.18 | 0.20 | 0.98 | 0.70 | 1.18 | 0.14 | 1.05 | 0.73 | 1.19 | 0.14 | | |
| B | 13 | 90523 | 12 | 63 | 0.40 | 0.40 | 1.20 | 0.80 | 0.98 | 0.70 | 1.18 | 0.25 | 0.98 | 0.70 | 1.18 | 0.25 | 0.98 | 0.70 | 1.18 | 0.13 | 1.05 | 0.73 | 1.18 | 0.13 | | |
| B | 14 | 90524 | 12 | 265 | 0.40 | 0.40 | 1.19 | 0.79 | 0.98 | 0.70 | 1.18 | 0.20 | 0.98 | 0.70 | 1.18 | 0.20 | 0.98 | 0.70 | 1.18 | 0.13 | 1.05 | 0.73 | 1.19 | 0.13 | | |
| B | 15 | 90525 | 12 | 84 | 0.40 | 0.40 | 1.18 | 0.78 | 0.98 | 0.70 | 1.18 | 0.19 | 0.98 | 0.70 | 1.18 | 0.19 | 0.98 | 0.70 | 1.18 | 0.13 | 1.05 | 0.74 | 1.18 | 0.13 | | |
| B | 16 | 90526 | 12 | 269 | 0.40 | 0.40 | 1.19 | 0.79 | 0.98 | 0.70 | 1.18 | 0.22 | 0.98 | 0.69 | 1.21 | 0.22 | 0.98 | 0.70 | 1.21 | 0.15 | 1.05 | 0.72 | 1.21 | 0.15 | | |
| B | 17 | 90527 | 12 | 264 | 0.40 | 0.40 | 1.19 | 0.79 | 0.98 | 0.70 | 1.18 | 0.20 | 0.98 | 0.70 | 1.18 | 0.20 | 0.98 | 0.70 | 1.18 | 0.13 | 1.05 | 0.73 | 1.18 | 0.13 | | |
| B | 18 | 90511 | 12 | 286 | 0.40 | 0.40 | 1.18 | 0.78 | 0.98 | 0.70 | 1.18 | 0.19 | 0.98 | 0.70 | 1.18 | 0.19 | 0.98 | 0.70 | 1.18 | 0.13 | 1.05 | 0.74 | 1.18 | 0.13 | | |
| B | 19 | 69295 | 15 | 487 | 0.40 | 0.29 | 2.16 | 1.76 | 0.98 | 0.47 | 2.16 | 1.17 | 0.98 | 0.41 | 2.73 | 1.75 | 0.98 | 0.41 | 2.73 | 1.75 | 0.98 | 0.43 | 2.73 | 1.68 | | |
| B | 20 | 69294 | 15 | 361 | 0.40 | 0.26 | 2.74 | 2.35 | 0.98 | 0.43 | 2.61 | 2.21 | 0.98 | 0.43 | 2.61 | 1.63 | 0.98 | 0.43 | 2.61 | 1.63 | 0.98 | 0.44 | 2.61 | 1.56 | | |
| B | 21 | 69293 | 15 | 173 | 0.40 | 0.26 | 2.61 | 2.21 | 0.98 | 0.43 | 2.61 | 2.21 | 0.98 | 0.38 | 3.14 | 2.16 | 0.98 | 0.38 | 3.14 | 2.16 | 0.98 | 0.40 | 3.14 | 2.09 | | |
| B | 22 | 69292 | 15 | 172 | 0.40 | 0.24 | 3.13 | 2.73 | 0.98 | 0.55 | 2.00 | 0.83 | 0.98 | 0.55 | 2.00 | 1.55 | 0.98 | 0.46 | 2 | | | | | | | |

Appendix A

Town of Marana Sewer Infrastructure Improvement Plan (IIP)

Sewer Pipe Capacity Tables

| Interceptor Branch | Segment | Model ID | Diameter (in) | Length (ft) |
|--------------------|---------|----------|---------------|-------------|
| B_NEW | 1 | 5106 | 15 | 350 |
| B_NEW | 2 | 5108 | 15 | 3,682 |
| B_NEW | 3 | 5110 | 15 | 780 |
| B_NEW | 4 | 5112 | 15 | 733 |
| B_NEW | 5 | 5114 | 15 | 625 |
| B_NEW | 6 | 5116 | 15 | 1,807 |
| B_NEW | 7 | 5118 | 15 | 1,181 |
| C | 1 | 987 | 12 | 459 |
| C | 2 | 600 | 12 | 428 |
| C | 3 | 608 | 12 | 251 |
| C | 4 | 607 | 12 | 364 |
| C | 5 | 606 | 12 | 396 |
| C | 6 | 605 | 12 | 217 |
| C | 7 | 604 | 12 | 301 |
| C | 8 | 603 | 12 | 246 |
| C | 9 | 602 | 12 | 335 |
| C | 10 | 601 | 12 | 146 |
| C | 11 | 901 | 15 | 23 |
| C | 12 | 887 | 15 | 489 |
| C | 13 | 895 | 15 | 259 |
| C | 14 | 894 | 15 | 48 |
| C | 15 | 618 | 15 | 214 |
| C | 16 | 893 | 15 | 263 |
| C | 17 | 892 | 15 | 263 |
| C | 18 | 891 | 15 | 262 |
| C | 19 | 890 | 15 | 359 |
| C | 20 | 889 | 15 | 268 |
| C | 21 | 877 | 15 | 363 |
| C | 22 | 696 | 15 | 191 |
| C | 23 | 983 | 15 | 500 |
| C | 24 | 587 | 15 | 397 |
| C | 25 | 586 | 15 | 251 |
| C | 26 | 585 | 15 | 270 |
| C | 27 | 584 | 15 | 271 |
| C | 28 | 546 | 15 | 27 |
| C | 29 | 583 | 15 | 224 |
| C | 30 | 582 | 15 | 500 |
| C | 31 | 581 | 15 | 250 |
| C | 1 | 5046 | 12 | 2,675 |
| D | 1 | 580 | 12 | 105 |
| D | 2 | 571 | 12 | 346 |
| D | 3 | 577 | 12 | 500 |
| D | 4 | 576 | 12 | 500 |
| D | 5 | 575 | 12 | 470 |
| D | 6 | 574 | 12 | 29 |
| D | 7 | 543 | 12 | 471 |
| D | 8 | 578 | 12 | 297 |
| D | 9 | 544 | 12 | 168 |
| D | 10 | 573 | 12 | 205 |
| D | 11 | 545 | 12 | 239 |
| D | 12 | 572 | 12 | 501 |
| D | 13 | 579 | 12 | 31 |
| D | 14 | 570 | 12 | 205 |
| D | 15 | 977 | 15 | 69 |
| D | 16 | 380 | 15 | 12 |
| D | 17 | 504 | 15 | 74 |
| D | 18 | 502 | 15 | 490 |
| D | 19 | 503 | 15 | 490 |
| D | 20 | 975 | 15 | 134 |
| D | 21 | 443 | 15 | 352 |
| D | 22 | 381 | 15 | 141 |

| Existing System | | | | |
|---------------------|------|-------------------------|---------------------------------|--|
| Existing Flow (MGD) | d/D | Existing Capacity (MGD) | Excess Capacity Available (MGD) | |
| 0.00 | 0.00 | 1.08 | 1.08 | |
| 0.00 | 0.00 | 1.00 | 1.00 | |
| 0.00 | 0.00 | 1.10 | 1.10 | |
| 0.00 | 0.00 | 1.01 | 1.01 | |
| 0.06 | 0.17 | 1.02 | 0.95 | |
| 0.06 | 0.17 | 1.00 | 0.95 | |
| 0.06 | 0.17 | 1.01 | 0.94 | |
| 0.06 | 0.17 | 1.02 | 0.95 | |
| 0.05 | 0.53 | 1.00 | 0.45 | |
| 0.05 | 0.53 | 1.02 | 0.46 | |
| 0.05 | 0.53 | 1.01 | 0.45 | |
| 0.05 | 0.53 | 1.00 | 0.45 | |
| 0.05 | 0.53 | 1.02 | 0.46 | |
| 0.05 | 0.50 | 1.01 | 0.50 | |
| 0.05 | 0.50 | 1.01 | 0.50 | |
| 0.05 | 0.53 | 1.00 | 0.45 | |
| 0.05 | 0.53 | 1.02 | 0.46 | |
| 0.05 | 0.53 | 1.01 | 0.45 | |
| 0.05 | 0.53 | 1.00 | 0.45 | |
| 0.05 | 0.53 | 1.02 | 0.46 | |
| 0.05 | 0.50 | 1.01 | 0.50 | |
| 0.05 | 0.50 | 1.01 | 0.50 | |
| 0.05 | 0.53 | 1.00 | 0.45 | |
| 0.05 | 0.53 | 1.02 | 0.46 | |
| 0.05 | 0.53 | 1.01 | 0.45 | |
| 0.05 | 0.53 | 1.00 | 0.45 | |
| 0.05 | 0.53 | 1.02 | 0.46 | |
| 0.05 | 0.50 | 1.01 | 0.50 | |
| 0.05 | 0.50 | 1.01 | 0.50 | |
| 0.05 | 0.53 | 1.00 | 0.45 | |
| 0.05 | 0.53 | 1.02 | 0.46 | |
| 0.05 | 0.53 | 1.01 | 0.45 | |
| 0.05 | 0.53 | 1.00 | 0.45 | |
| 0.05 | 0.53 | 1.02 | 0.46 | |
| 0.05 | 0.50 | 1.01 | 0.50 | |
| 0.05 | 0.50 | 1.01 | 0.50 | |
| 0.05 | 0.53 | 1.00 | 0.45 | |
| 0.05 | 0.53 | 1.02 | 0.46 | |
| 0.05 | 0.53 | 1.01 | 0.45 | |
| 0.05 | 0.53 | 1.00 | 0.45 | |
| 0.05 | 0.53 | 1.02 | 0.46 | |
| 0.05 | 0.50 | 1.01 | 0.50 | |
| 0.05 | 0.50 | 1.01 | 0.50 | |
| 0.05 | 0.53 | 1.00 | 0.45 | |
| 0.05 | 0.53 | 1.02 | 0.46 | |
| 0.05 | 0.53 | 1.01 | 0.45 | |
| 0.05 | 0.53 | 1.00 | 0.45 | |
| 0.05 | 0.53 | 1.02 | 0.46 | |
| 0.05 | 0.50 | 1.01 | 0.50 | |
| 0.05 | 0.50 | 1.01 | 0.50 | |
| 0.05 | 0.53 | 1.00 | 0.45 | |
| 0.05 | 0.53 | 1.02 | 0.46 | |
| 0.05 | 0.53 | 1.01 | 0.45 | |
| 0.05 | 0.53 | 1.00 | 0.45 | |
| 0.05 | 0.53 | 1.02 | 0.46 | |
| 0.05 | 0.50 | 1.01 | 0.50 | |
| 0.05 | 0.50 | 1.01 | 0.50 | |
| 0.05 | 0.53 | 1.00 | 0.45 | |
| 0.05 | 0.53 | 1.02 | 0.46 | |
| 0.05 | 0.53 | 1.01 | 0.45 | |
| 0.05 | 0.53 | 1.00 | 0.45 | |
| 0.05 | 0.53 | 1.02 | 0.46 | |
| 0.05 | 0.50 | 1.01 | 0.50 | |
| 0.05 | 0.50 | 1.01 | 0.50 | |
| 0.05 | 0.53 | 1.00 | 0.45 | |
| 0.05 | 0.53 | 1.02 | 0.46 | |
| 0.05 | 0.53 | 1.01 | 0.45 | |
| 0.05 | 0.53 | 1.00 | 0.45 | |
| 0.05 | 0.53 | 1.02 | 0.46 | |
| 0.05 | 0.50 | 1.01 | 0.50 | |
| 0.05 | 0.50 | 1.01 | 0.50 | |
| 0.05 | 0.53 | 1.00 | 0.45 | |
| 0.05 | 0.53 | 1.02 | 0.46 | |
| 0.05 | 0.53 | 1.01 | 0.45 | |
| 0.05 | 0.53 | 1.00 | 0.45 | |
| 0.05 | 0.53 | 1.02 | 0.46 | |
| 0.05 | 0.50 | 1.01 | 0.50 | |
| 0.05 | 0.50 | 1.01 | 0.50 | |
| 0.05 | 0.53 | 1.00 | 0.45 | |
| 0.05 | 0.53 | 1.02 | 0.46 | |
| 0.05 | 0.53 | 1.01 | 0.45 | |
| 0.05 | 0.53 | 1.00 | 0.45 | |
| 0.05 | 0.53 | 1.02 | 0.46 | |
| 0.05 | 0.50 | 1.01 | 0.50 | |
| 0.05 | 0.50 | 1.01 | 0.50 | |
| 0.05 | 0.53 | 1.00 | 0.45 | |
| 0.05 | 0.53 | 1.02 | 0.46 | |
| 0.05 | 0.53 | 1.01 | 0.45 | |
| 0.05 | 0.53 | 1.00 | 0.45 | |
| 0.05 | 0.53 | 1.02 | 0.46 | |
| 0.05 | 0.50 | 1.01 | 0.50 | |
| 0.05 | 0.50 | 1.01 | 0.50 | |
| 0.05 | 0.53 | 1.00 | 0.45 | |
| 0.05 | 0.53 | 1.02 | 0.46 | |
| 0.05 | 0.53 | 1.01 | 0.45 | |
| 0.05 | 0.53 | 1.00 | 0.45 | |
| 0.05 | 0.53 | 1.02 | 0.46 | |
| 0.05 | 0.50 | 1.01 | 0.50 | |
| 0.05 | 0.50 | 1.01 | 0.50 | |
| 0.05 | 0.53 | 1.00 | 0.45 | |
| 0.05 | 0.53 | 1.02 | 0.46 | |
| 0.05 | 0.53 | 1.01 | 0.45 | |
| 0.05 | 0.53 | 1.00 | 0.45 | |
| 0.05 | 0.53 | 1.02 | | |

Appendix A

Town of Marana Sewer Infrastructure Improvement Plan (IIP)

Sewer Pipe Capacity Tables

| Interceptor Branch | Segment | Model ID | Diameter (in) | Length (ft) |
|--------------------|---------|----------|---------------|-------------|
| E | 1 | 287 | 12 | 85 |
| E | 2 | 403 | 12 | 500 |
| E | 3 | 402 | 12 | 500 |
| E | 4 | 409 | 12 | 500 |
| E | 5 | 408 | 12 | 500 |
| E | 6 | 407 | 12 | 500 |
| E | 7 | 406 | 12 | 500 |
| E | 8 | 405 | 12 | 487 |
| E | 9 | 404 | 12 | 39 |
| E | 10 | 4004 | 12 | 24 |
| E | 11 | 400 | 12 | 23 |
| E | 12 | 965 | 12 | 337 |
| E | 13 | 86 | 12 | 158 |
| E | 14 | 91 | 12 | 355 |
| E | 15 | 90 | 12 | 465 |
| E | 16 | 89 | 12 | 299 |
| E | 17 | 88 | 12 | 349 |
| E | 18 | 87 | 12 | 335 |
| E | 19 | 96 | 12 | 319 |
| E | 20 | 81 | 15 | 288 |
| E | 21 | 80 | 15 | 84 |
| F | 1 | 79 | 15 | 448 |
| F | 2 | 78 | 15 | 501 |
| F | 3 | 77 | 15 | 500 |
| F | 4 | 76 | 15 | 500 |
| F | 5 | 75 | 15 | 501 |
| F | 6 | 74 | 15 | 500 |
| F | 7 | 73 | 15 | 499 |
| F | 8 | 72 | 15 | 500 |
| F | 9 | 71 | 15 | 499 |
| F | 10 | 962 | 15 | 476 |
| G | 1 | 970 | 18 | 563 |
| G | 2 | 461 | 18 | 500 |
| G | 3 | 460 | 18 | 477 |
| G | 4 | 5064 | 18 | 80 |
| G | 1 | 459 | 18 | 57 |
| G_NEW | 2 | 5120 | 15 | 1,369 |
| G_NEW | 3 | 5122 | 15 | 516 |
| H | 1 | 978 | 15 | 499 |
| H | 2 | 458 | 15 | 387 |
| H | 3 | 457 | 15 | 394 |
| H | 4 | 456 | 21 | 337 |
| H | 5 | 484 | 12 | 48 |
| I | 1 | 499 | 12 | 349 |
| I | 2 | 498 | 12 | 211 |
| I | 3 | 487 | 15 | 273 |
| I | 4 | 497 | 15 | 400 |
| I | 5 | 496 | 15 | 164 |
| I | 6 | 495 | 15 | 380 |
| I | 7 | 494 | 15 | 406 |
| I | 8 | 493 | 15 | 171 |
| I | 9 | 492 | 15 | 219 |
| I | 10 | 491 | 15 | 500 |
| I | 11 | 490 | 15 | 500 |
| I | 12 | 489 | 15 | 131 |
| I | 13 | 488 | 15 | 40 |
| I_NEW | 1 | 500 | 21 | 77 |
| I_NEW | 2 | 455 | 21 | 318 |
| I_NEW | 3 | 5018 | 21 | 815 |
| I_NEW | 4 | 5014 | 21 | 2,028 |
| I_NEW | 5 | 5012 | 21 | 1,740 |
| J | 1 | 382 | 15 | 359 |
| J | 2 | 383 | 15 | 400 |
| J | 3 | 384 | 15 | 500 |
| J | 4 | 385 | 15 | 500 |
| J | 5 | 442 | 15 | 500 |
| J | 6 | 441 | 15 | 500 |
| J | 7 | 440 | 15 | 500 |
| J | 8 | 439 | 15 | 260 |
| J | 9 | 967 | 15 | 492 |
| J | 10 | 100 | 15 | 500 |
| J | 11 | 99 | 15 | 500 |
| J | 12 | 98 | 15 | 500 |
| J | 13 | 97 | 18 | 537 |
| J | 14 | 963 | 18 | 139 |
| J | 15 | 39 | 18 | 44 |

| Existing System | | | | | 2022 to 2027 (5 Year) without Improvements | | | | 2022 to 2027 (5 Year) with Improvements | | | | 2027 to 2032 (10 Year) without Improvements | | | | 2027 to 2032 (10 Year) with Improvements | | | | Improvement Notes | | | |
|----------------------|------|--------------------------|---------------------------------|----------------------|--|--------------------------|---------------------------------|----------------------|---|--------------------------|---------------------------------|----------------------|---|--------------------------|---------------------------------|----------------------|--|--------------------------|---------------------------------|-----------------------------------|-------------------|--|--|--|
| Projected Flow (MGD) | d/D | Projected Capacity (MGD) | Excess Capacity Available (MGD) | Projected Flow (MGD) | d/D | Projected Capacity (MGD) | Excess Capacity Available (MGD) | Projected Flow (MGD) | d/D | Projected Capacity (MGD) | Excess Capacity Available (MGD) | Projected Flow (MGD) | d/D | Projected Capacity (MGD) | Excess Capacity Available (MGD) | Projected Flow (MGD) | d/D | Projected Capacity (MGD) | Excess Capacity Available (MGD) | Improvement Notes | | | | |
| 0.19 | 0.30 | 0.97 | 0.78 | 0.47 | 0.45 | 1.11 | 0.64 | 0.47 | 0.45 | 1.11 | 0.64 | 0.39 | 0.41 | 1.11 | 0.72 | 0.27 | 0.34 | 1.11 | 0.84 | Replaced by Improvement | | | | |
| 0.19 | 0.30 | 1.01 | 0.81 | 0.52 | 0.51 | 1.01 | 0.49 | 0.52 | 0.51 | 1.01 | 0.49 | 0.46 | 0.48 | 1.01 | 0.54 | 0.33 | 0.39 | 1.01 | 0.68 | New Pipe - 2022 to 2027 (5 Year) | | | | |
| 0.19 | 0.30 | 1.01 | 0.81 | 0.52 | 0.51 | 1.01 | 0.49 | 0.52 | 0.51 | 1.01 | 0.49 | 0.46 | 0.48 | 1.01 | 0.54 | 0.33 | 0.39 | 1.01 | 0.68 | New Pipe - 2027 to 2032 (10 Year) | | | | |
| 0.19 | 0.30 | 1.01 | 0.81 | 0.52 | 0.51 | 1.01 | 0.49 | 0.57 | 0.54 | 1.01 | 0.43 | 0.57 | 0.54 | 1.01 | 0.43 | 0.45 | 0.47 | 1.01 | 0.56 | New Pipe - 2027 to 2032 (10 Year) | | | | |
| 0.19 | 0.30 | 1.01 | 0.81 | 0.57 | 0.54 | 1.01 | 0.43 | 0.57 | 0.54 | 1.01 | 0.43 | 0.58 | 0.54 | 1.01 | 0.43 | 0.45 | 0.47 | 1.01 | 0.56 | Replaced by Improvement | | | | |
| 0.19 | 0.30 | 1.01 | 0.81 | 0.57 | 0.54 | 1.01 | 0.43 | 0.58 | 0.56 | 1.38 | 0.80 | 0.58 | 0.56 | 0.98 | 0.39 | 0.45 | 0.47 | 1.01 | 0.56 | New Pipe - 2022 to 2027 (5 Year) | | | | |
| 0.19 | 0.30 | 1.01 | 0.81 | 0.58 | 0.56 | 0.98 | 0.40 | 0.58 | 0.56 | 0.98 | 0.40 | 0.60 | 0.45 | 1.41 | 0.81 | 0.46 | 0.48 | 0.98 | 0.52 | New Pipe - 2022 to 2027 (5 Year) | | | | |
| 0.19 | 0.30 | 1.01 | 0.81 | 0.60 | 0.53 | 1.08 | 0.48 | 0.60 | 0.53 | 1.08 | 0.48 | 0.65 | 0.62 | 0.93 | 0.26 | 0.48 | 0.47 | 1.08 | 0.60 | Replaced by Improvement | | | | |
| 0.19 | 0.30 | 1.01 | 0.81 | 0.65 | 0.62 | 0.93 | 0.28 | 0.66 | 0.63 | 0.92 | 0.25 | 0.66 | 0.63 | 0.92 | 0.25 | 0.55 | 0.56 | 0.92 | 0.37 | New Pipe - 2022 to 2027 (5 Year) | | | | |
| 0.19 | 0.30 | 1.01 | 0.81 | 0.66 | 0.63 | 0.92 | 0.25 | 0.66 | 0.63 | 0.92 | 0.26 | 0.70 | 0.66 | 0.92 | 0.21 | 0.55 | 0.56 | 0.92 | 0.37 | New Pipe - 2022 to 2027 (5 Year) | | | | |
| 0.19 | 0.30 | 1.01 | 0.81 | 0.72 | 0.66 | 0.94 | 0.22 | 0.72 | 0.67 | 0.92 | 0.20 | 0.72 | 0.67 | 0.92 | 0.20 | 0.55 | 0.56 | 0.92 | 0.37 | New Pipe - 2022 to 2027 (5 Year) | | | | |
| 0.19 | 0.30 | 1.01 | 0.81 | 0.72 | 0.67 | 0.92 | 0.20 | 0.73 | 0.68 | 0.91 | 0.18 | 0.73 | 0.68 | 0.91 | 0.18 | 0.55 | 0.56 | 0.92 | 0.37 | New Pipe - 2022 to 2027 (5 Year) | | | | |
| 0.19 | 0.30 | 1.01 | 0.81 | 0.74 | 0.46 | 1.71 | 0.97 | 0.74 | 0.47 | 1.66 | 0.92 | 0.74 | 0.47 | 1.66 | 0.92 | 0.55 | 0.56 | 1.71 | 0.90 | Replaced by Improvement | | | | |
| 0.19 | 0.30 | 1.01 | 0.81 | 0.74 | 0.47 | 1.66 | 0.92 | 0.74 | 0.47 | 1.66 | 0.92 | 0.74 | 0.49 | 1.66 | 0.84 | 0.55 | 0.56 | 1.66 | 0.97 | New Pipe - 2022 to 2027 (5 Year) | | | | |
| 0.19 | 0.30 | 1.01 | 0.81 | 0.74 | 0.47 | 1.66 | 0.92 | 0.74 | 0.47 | 1.66 | 0.92 | 0.74 | 0.49 | 1.71 | 0.94 | 0.55 | 0.56 | 1.74 | 0.97 | New Pipe - 2022 to 2027 (5 Year) | | | | |
| 0.19 | 0.30 | 1.01 | 0.81 | 0.74 | 0.47 | 1.66 | 0.92 | 0.74 | 0.47 | 1.66 | 0.92 | 0.74 | 0.49 | 1.71 | 0.94 | 0.55 | 0.56 | 1.74 | 0.97 | New Pipe - 2022 to 2027 (5 Year) | | | | |
| 0.19 | 0.30 | 1.01 | 0.81 | 0.74 | 0.47 | 1.66 | 0.92 | 0.74 | 0.47 | 1 | | | | | | | | | | | | | | |

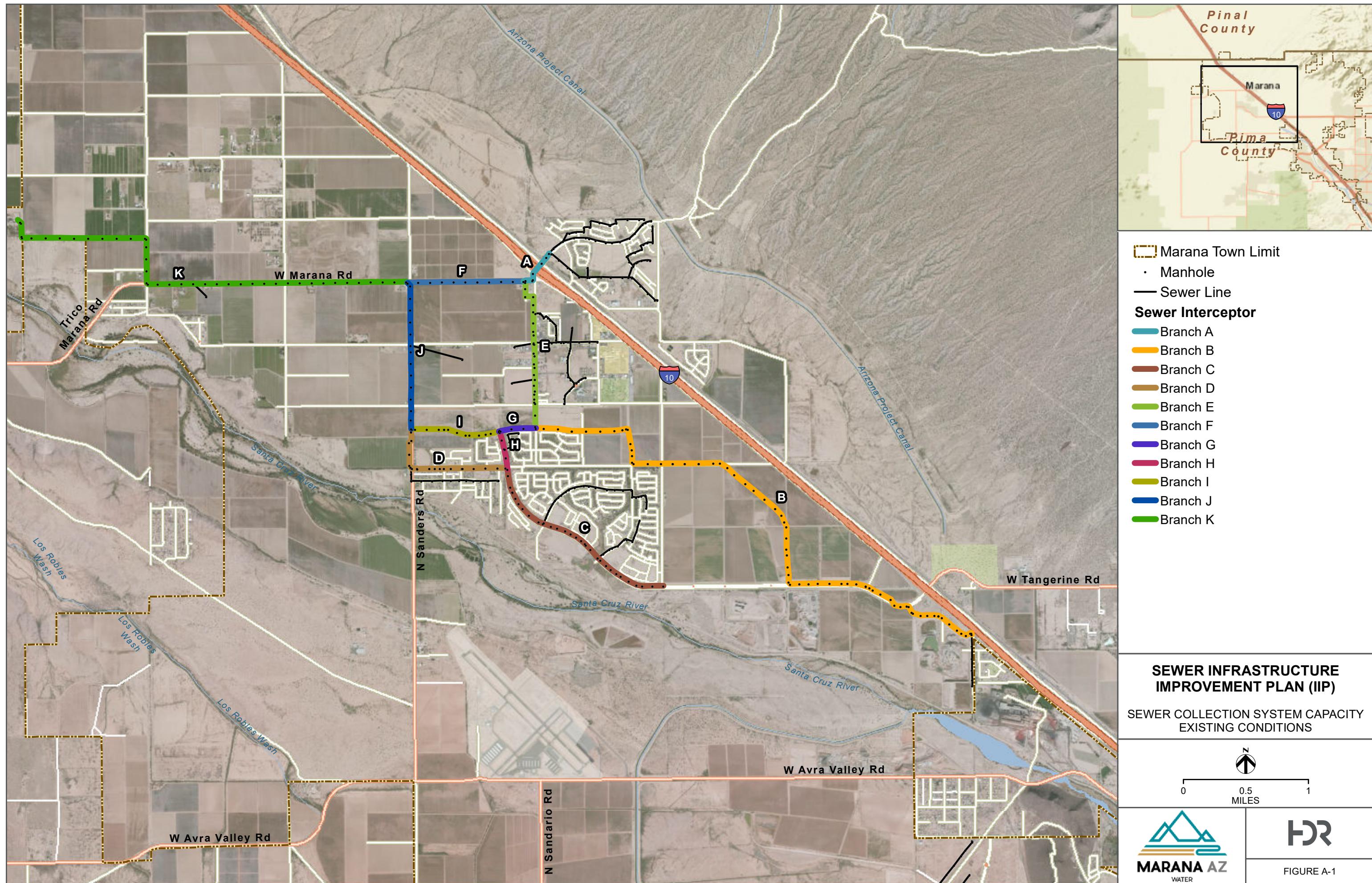
Appendix A

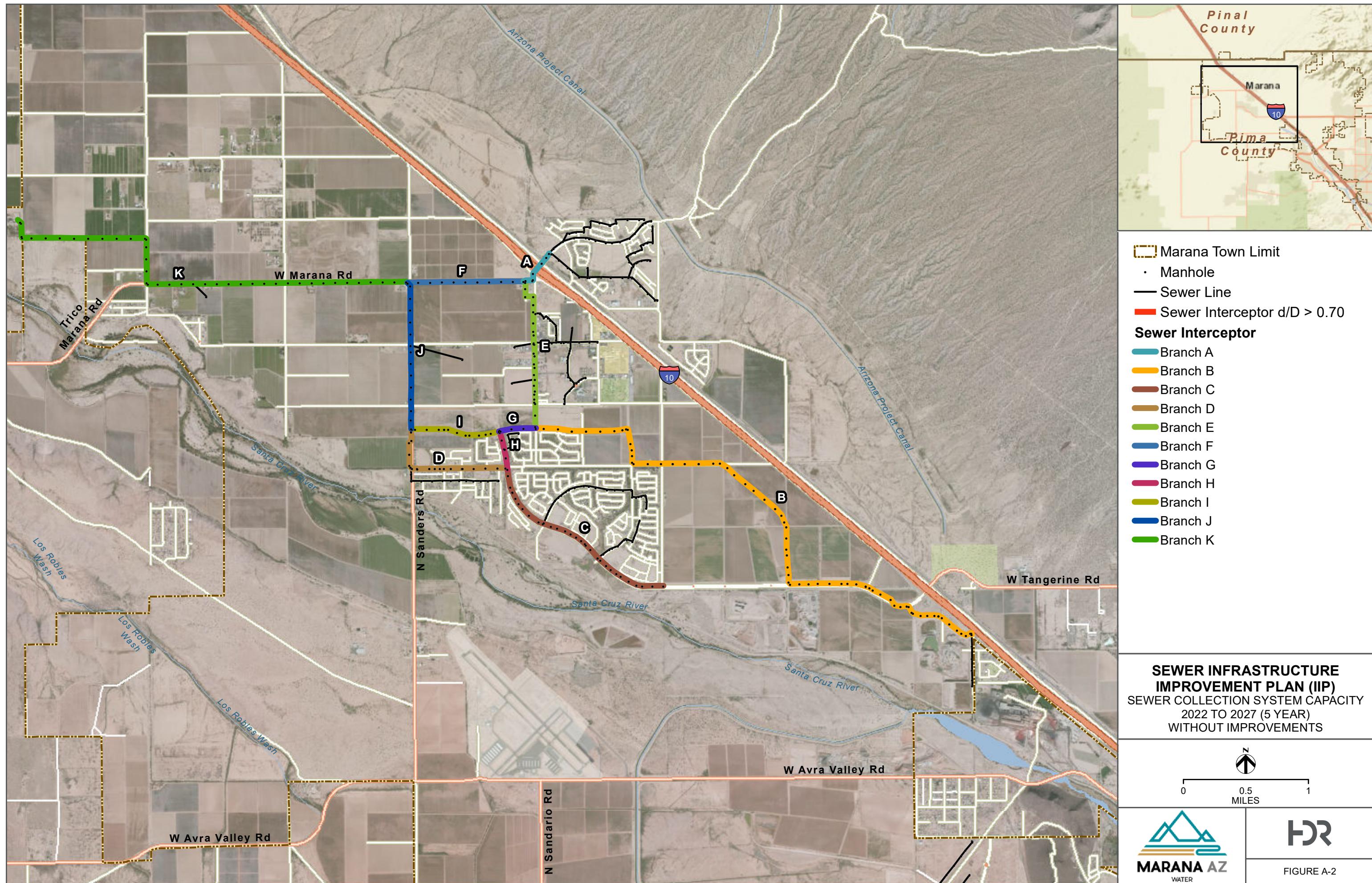
**Town of Marana
Infrastructure Improvement Plan (IIP)**

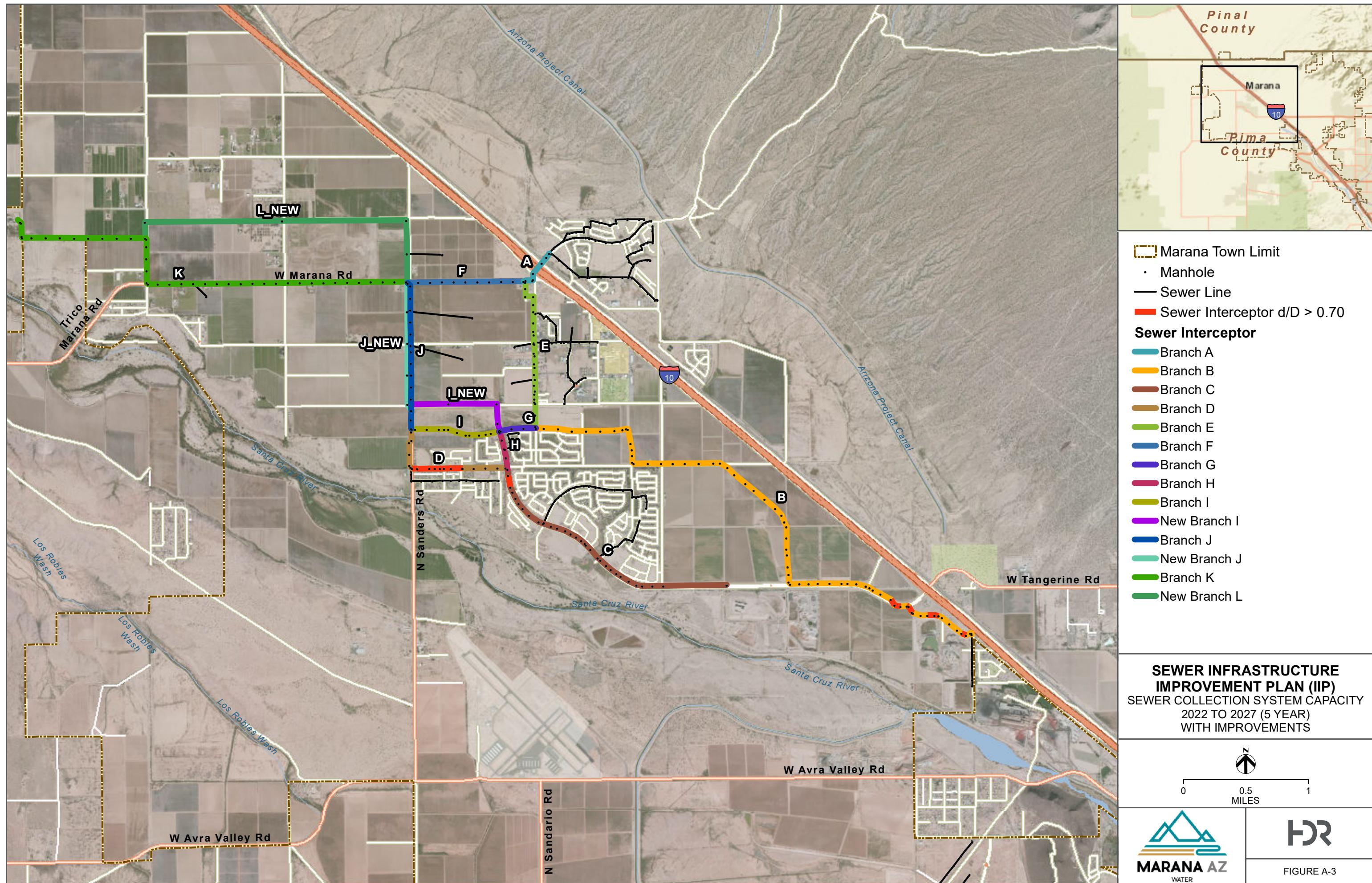
Water Pipe Capacity Tables

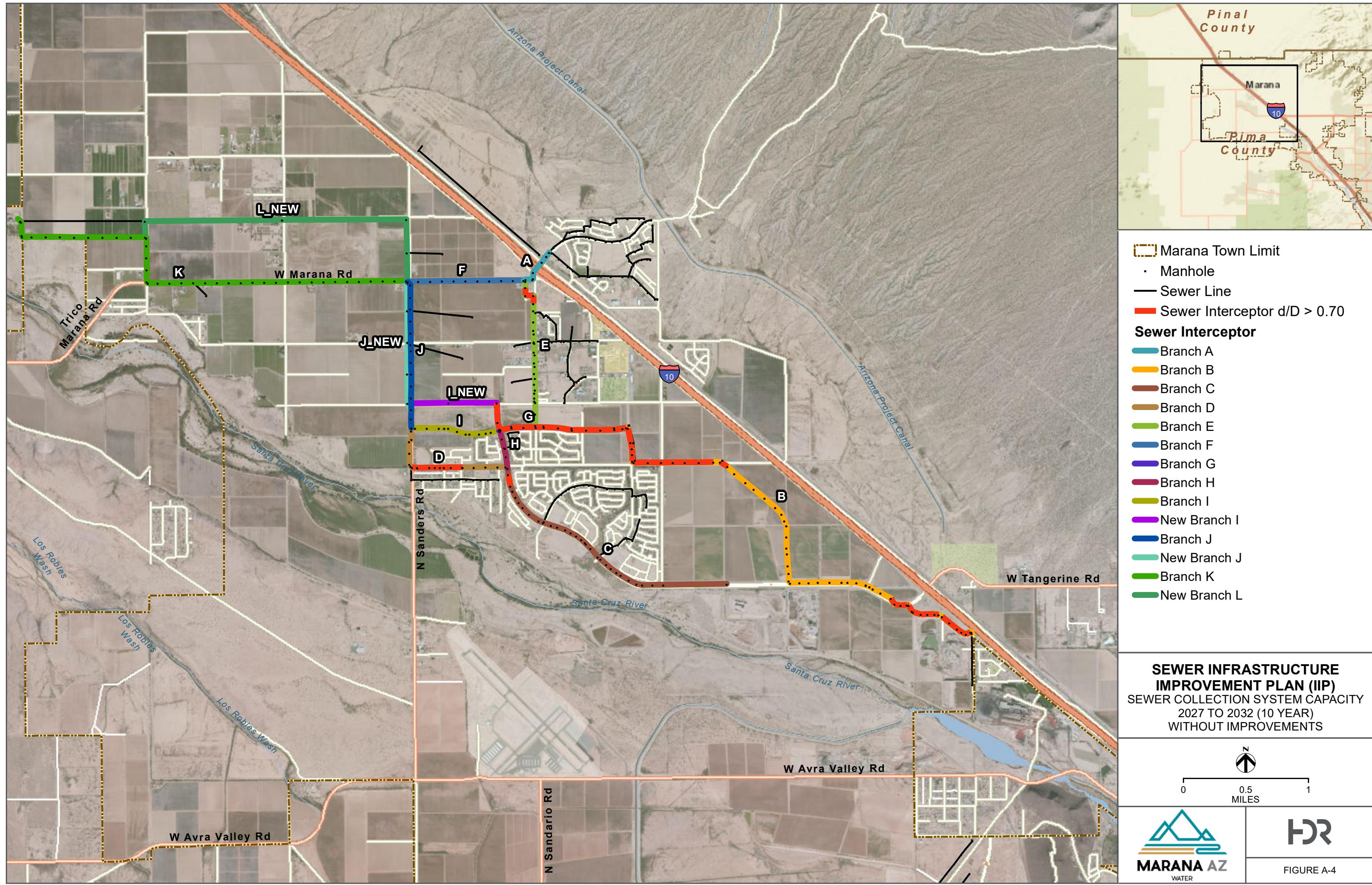
| Interceptor Branch | Segment | Model ID | Existing System | | 2022 to 2027 (5 Year) without Improvements | | | 2022 to 2027 (5 Year) with Improvements | | | 2022 to 2032 (10 Year) without Improvements | | | 2022 to 2032 (10 Year) with Improvements | | | | | | |
|--------------------|---------|----------|-----------------|-------------|--|------|-------------------------|---|----------------------|------|---|---------------------------------|----------------------|--|--------------------------|---------------------------------|----------------------|------|--------------------------|---------------------------------|
| | | | Diameter (in) | Length (ft) | Existing Flow (MGD) | d/D | Existing Capacity (MGD) | Excess Capacity Available (MGD) | Projected Flow (MGD) | d/D | Projected Capacity (MGD) | Excess Capacity Available (MGD) | Projected Flow (MGD) | d/D | Projected Capacity (MGD) | Excess Capacity Available (MGD) | Projected Flow (MGD) | d/D | Projected Capacity (MGD) | Excess Capacity Available (MGD) |
| J_NEW | 1 | 5104 | 27 | 308 | | | | | | | | | | | | | | | | |
| J_NEW | 2 | 5010 | 27 | 1,245 | | | | | | | | | | | | | | | | |
| J_NEW | 3 | 5008 | 27 | 1,342 | | | | | | | | | | | | | | | | |
| J_NEW | 4 | 5006 | 27 | 1,338 | | | | | | | | | | | | | | | | |
| J_NEW | 5 | 5004 | 27 | 1,229 | | | | | | | | | | | | | | | | |
| K | 1 | 61 | 18 | 49 | 1.64 | 0.61 | 2.38 | 0.74 | 4.33 | 1.00 | 2.92 | -1.40 | 1.52 | 0.51 | 2.92 | 1.40 | 2.16 | 0.64 | 2.92 | 0.77 |
| K | 2 | 50 | 18 | 494 | 1.64 | 0.53 | 2.94 | 1.30 | 4.33 | 1.00 | 2.89 | -1.44 | 1.64 | 0.54 | 2.89 | 1.24 | 2.32 | 0.68 | 2.89 | 0.57 |
| K | 3 | 59 | 18 | 500 | 1.64 | 0.52 | 3.04 | 1.41 | 4.33 | 1.00 | 3.04 | -1.29 | 1.64 | 0.52 | 3.04 | 1.40 | 2.32 | 0.65 | 3.04 | 0.72 |
| K | 4 | 58 | 18 | 500 | 1.64 | 0.51 | 3.21 | 1.57 | 4.33 | 1.00 | 3.06 | -1.27 | 4.33 | 1.00 | 3.03 | -1.30 | 1.64 | 0.52 | 3.06 | 1.42 |
| K | 5 | 57 | 18 | 500 | 1.64 | 0.54 | 2.87 | 1.24 | 4.33 | 1.00 | 3.12 | -1.21 | 1.64 | 0.52 | 3.03 | 1.39 | 1.64 | 0.52 | 3.12 | 0.80 |
| K | 6 | 56 | 18 | 500 | 1.64 | 0.49 | 3.40 | 1.77 | 4.33 | 1.00 | 3.04 | -1.41 | 4.33 | 1.00 | 3.06 | -1.27 | 2.32 | 0.65 | 3.04 | 0.72 |
| K | 7 | 55 | 18 | 500 | 1.64 | 0.52 | 3.04 | 1.41 | 4.33 | 1.00 | 3.06 | -1.29 | 1.64 | 0.52 | 3.04 | 1.40 | 2.32 | 0.65 | 3.06 | 0.74 |
| K | 8 | 54 | 18 | 500 | 1.64 | 0.54 | 2.89 | 1.25 | 4.33 | 1.00 | 3.06 | -1.27 | 1.64 | 0.52 | 3.06 | 1.42 | 2.32 | 0.66 | 2.98 | 0.66 |
| K | 9 | 53 | 18 | 500 | 1.64 | 0.51 | 3.13 | 1.50 | 4.33 | 1.00 | 2.98 | -1.35 | 4.33 | 1.00 | 2.87 | -1.46 | 1.64 | 0.54 | 2.87 | 0.55 |
| K | 10 | 52 | 18 | 500 | 1.64 | 0.56 | 2.71 | 1.07 | 4.33 | 1.00 | 3.04 | -1.29 | 1.64 | 0.52 | 3.04 | 1.40 | 2.32 | 0.65 | 3.04 | 0.72 |
| K | 11 | 51 | 18 | 500 | 1.64 | 0.52 | 3.04 | 1.41 | 4.33 | 1.00 | 3.04 | -1.29 | 1.64 | 0.52 | 3.04 | 1.42 | 2.32 | 0.66 | 3.02 | 0.70 |
| K | 12 | 961 | 18 | 500 | 1.64 | 0.52 | 3.04 | 1.41 | 4.33 | 1.00 | 3.04 | -1.28 | 1.64 | 0.52 | 3.04 | 1.40 | 2.32 | 0.65 | 3.04 | 0.72 |
| K | 13 | 36 | 18 | 500 | 1.64 | 0.55 | 2.81 | 1.17 | 4.33 | 1.00 | 3.02 | -1.31 | 1.64 | 0.53 | 3.02 | 1.38 | 2.32 | 0.66 | 3.02 | 0.70 |
| K | 14 | 35 | 18 | 500 | 1.64 | 0.53 | 3.01 | 1.38 | 4.33 | 1.00 | 2.97 | -1.36 | 4.33 | 1.00 | 3.29 | -1.04 | 1.64 | 0.50 | 3.29 | 1.65 |
| K | 15 | 34 | 18 | 500 | 1.64 | 0.50 | 3.29 | 1.66 | 4.33 | 1.00 | 3.29 | -1.04 | 4.33 | 1.00 | 3.04 | -1.29 | 1.64 | 0.52 | 3.04 | 1.40 |
| K | 16 | 33 | 18 | 500 | 1.64 | 0.52 | 3.04 | 1.41 | 4.33 | 1.00 | 3.21 | -1.12 | 1.64 | 0.51 | 3.21 | 1.56 | 2.32 | 0.63 | 3.21 | 0.89 |
| K | 17 | 32 | 18 | 500 | 1.64 | 0.51 | 3.21 | 1.57 | 4.33 | 1.00 | 2.88 | -1.45 | 4.33 | 1.00 | 3.04 | -1.28 | 2.32 | 0.68 | 2.88 | 0.56 |
| K | 18 | 31 | 18 | 500 | 1.64 | 0.54 | 2.87 | 1.24 | 4.33 | 1.00 | 3.04 | -1.29 | 4.33 | 1.00 | 3.04 | -1.29 | 1.64 | 0.52 | 3.04 | 0.72 |
| K | 19 | 30 | 18 | 500 | 1.64 | 0.52 | 3.04 | 1.41 | 4.33 | 1.00 | 3.04 | -1.29 | 4.33 | 1.00 | 3.04 | -1.28 | 2.32 | 0.65 | 3.04 | 0.72 |
| K | 20 | 29 | 18 | 500 | 1.64 | 0.52 | 3.04 | 1.41 | 4.33 | 1.00 | 3.04 | -1.28 | 4.33 | 1.00 | 3.09 | -1.24 | 1.64 | 0.52 | 3.09 | 0.77 |
| K | 21 | 28 | 18 | 500 | 1.64 | 0.50 | 3.28 | 1.64 | 4.33 | 1.00 | 3.15 | -1.18 | 1.64 | 0.51 | 3.15 | 1.51 | 2.32 | 0.64 | 3.15 | 0.83 |
| K | 22 | 27 | 18 | 500 | 1.64 | 0.55 | 2.79 | 1.15 | 4.33 | 1.00 | 3.05 | -1.27 | 4.33 | 1.00 | 3.05 | -1.41 | 1.64 | 0.52 | 3.05 | 0.73 |
| K | 23 | 26 | 18 | 417 | 1.64 | 0.52 | 3.05 | 1.42 | 4.33 | 1.00 | 3.01 | -1.32 | 4.33 | 1.00 | 3.01 | -1.37 | 1.64 | 0.53 | 3.01 | 0.69 |
| K | 24 | 25 | 18 | 72 | 1.64 | 0.53 | 3.01 | 1.38 | 4.33 | 1.00 | 3.05 | -1.28 | 4.33 | 1.00 | 3.05 | -1.41 | 1.64 | 0.52 | 3.05 | 0.73 |
| K | 25 | 23 | 18 | 529 | 1.64 | 0.52 | 3.05 | 1.41 | 4.33 | 1.00 | 3.04 | -1.28 | 4.33 | 1.00 | 3.04 | -1.28 | 1.64 | 0.52 | 3.04 | 0.72 |
| K | 26 | 22 | 18 | 500 | 1.64 | 0.52 | 3.04 | 1.41 | 4.33 | 1.00 | 3.04 | -1.28 | 4.33 | 1.00 | 3.04 | -1.28 | 1.64 | 0.52 | 3.04 | 0.72 |
| K | 27 | 21 | 18 | 500 | 1.64 | 0.52 | 3.04 | 1.41 | 4.33 | 1.00 | 3.04 | -1.28 | 4.33 | 1.00 | 3.04 | -1.28 | 1.64 | 0.52 | 3.04 | 0.72 |
| K | 28 | 20 | 18 | 330 | 1.64 | 0.52 | 3.04 | 1.41 | 4.33 | 1.00 | 3.04 | -1.29 | 4.33 | 1.00 | 3.04 | -1.29 | 1.64 | 0.52 | 3.04 | 0.72 |
| K | 29 | 960 | 18 | 62 | 1.64 | 0.32 | 7.20 | 5.56 | 4.33 | 0.56 | 7.16 | 2.83 | 1.64 | 0.33 | 7.16 | 5.52 | 2.32 | 0.39 | 7.16 | 4.84 |
| K | 30 | 2 | 24 | 600 | 1.64 | 0.37 | 5.58 | 3.95 | 4.37 | 0.67 | 5.59 | 1.21 | 4.39 | 0.67 | 5.59 | 1.20 | 4.49 | 0.68 | 5.59 | 1.10 |
| K | 31 | 9 | 24 | 600 | 1.64 | 0.37 | 5.58 | 3.95 | 4.37 | 0.67 | 5.58 | 1.20 | 4.39 | 0.67 | 5.58 | 1.19 | 4.49 | 0.68 | 5.58 | 1.09 |
| K | 32 | 8 | 24 | 600 | 1.64 | 0.37 | 5.58 | 3.95 | 4.37 | 0.67 | 5.59 | 1.21 | 4.39 | 0.67 | 5.59 | 1.20 | 4.49 | 0.67 | 5.64 | 1.16 |
| K | 33 | 7 | 24 | 600 | 1.64 | 0.37 | 5.64 | 4.01 | 4.37 | 0.66 | 5.64 | 1.27 | 4.39 | 0.66 | 5.64 | 1.25 | 4.49 | 0.68 | 5.58 | 1.10 |
| K | 34 | 6 | 24 | 600 | 1.64 | 0.37 | 5.58 | 3.95 | 4.37 | 0.67 | 5.58 | 1.21 | 4.39 | 0.67 | 5.58 | 1.20 | 4.49 | 0.68 | 5.59 | 1.10 |
| K | 35 | 5 | 24 | 600 | 1.64 | 0.37 | 5.58 | 3.95 | 4.37 | 0.67 | 5.59 | 1.21 | 4.39 | 0.67 | 5.59 | 1.20 | 4.49 | 0.68 | 5.59 | 1.10 |
| K | 36 | 4 | 24 | 600 | 1.64 | 0.37 | 5.58 | 3.95 | 4.37 | 0.67 | 5.58 | 1.21 | 4.39 | 0.67 | 5.58 | 1.19 | 4.49 | 0.68 | 5.58 | 1.09 |
| K | 37 | 3 | 24 | 600 | 1.64 | 0.37 | 5.58 | 3.95 | 4.37 | 0.67 | 5.59 | 1.22 | 4.39 | 0.67 | 5.59 | 1.20 | 4.49 | 0.68 | 5.59 | 1.10 |
| K | 38 | 959 | 24 | 406 | 1.64 | 0.37 | 5.59 | 3.95 | 4.37 | 0.67 | 5.59 | 1.22 | 4.39 | 0.67 | 5.59 | 1.20 | 4.49 | 0.68 | 5.59 | 1.10 |
| K | 39 | 14 | 24 | 49 | 1.64 | 0.37 | 5.51 | 3.88 | 4.37 | 0.68 | 5.41 | 1.04 | 4.39 | 0.68 | 5.41 | 1.02 | 4.49 | 0.69 | 5.41 | 0.92 |
| K | 40 | 16 | 24 | 583 | 1.64 | 0.37 | 5.56 | 3.93 | 4.37 | 0.67 | 5.57 | 1.20 | 4.39 | 0.67 | 5.57 | 1.18 | 4.49 | 0.68 | 5.57 | 1.08 |
| K | 41 | 15 | 24 | 54 | 1.64 | 0.37 | 5.63 | 3.99 | 4.37 | 0.66 | 5.67 | 1.30 | 4.39 | 0.66 | 5.67 | 1.28 | 4.49 | 0.67 | 5.67 | 1.19 |
| K | 42 | 4002 | 24 | 5 | 1.64 | 0.17 | 25.39 | 23.76 | 4.37 | 0.38 | 14.42 | 10.05 | 4.39 | 0.38 | 14.42 | 10.03 | 6.58 | 0.47 | 14.42 | 7.84 |
| L_NEW | 1 | 5058 | 27 | 1,211 | | | | | | | | | | | | | 4.01 | 0.54 | 7.06 | 3.06 |
| L_NEW | 2 | 5060 | 27 | 1,424 | | | | | | | | | | | | | 4.25 | 0.54 | 7.52 | 3.27 |
| L_NEW | 3 | 5056 | 27 | 5,243 | | | | | | | | | | | | | 4.25 | 0.53 | 7.59 | 3.34 |
| L_NEW | 4 | 5054 | 27 | 5,807 | | | | | | | | | | | | | 4.55 | 0.46 | 10.70 | 6.15 |
| L_NEW | 5 | 5052 | 27 | 5,257 | | | | | | | | | | | | | 2.27 | 0.33 | 9.43 | 7.16 |
| I_NEW | 1 | 5100 | 27 | 684 | | | | | | | | | | | | | 2.28 | 0.33 | 9.46 | 7.19 |

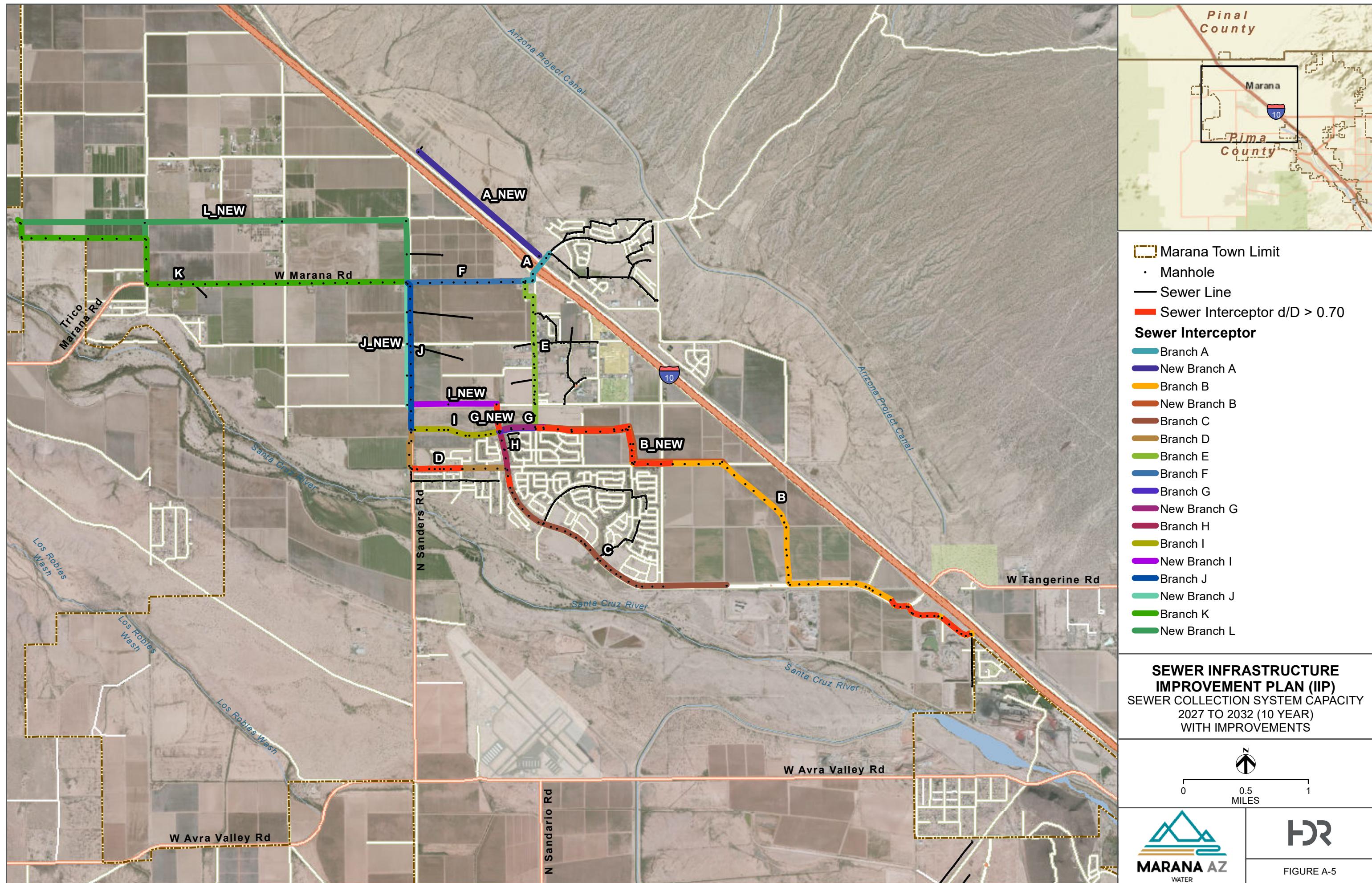
Appendix B: Sewer Collection System Capacity Figures











Appendix C: CIP Cost Tables

Town of Marana Sewer IIP CIP Cost Summary

| Year | ID | Construction Costs | | | Mob/ Demob 1% | General Conditions 8% | Overhead 10% | Contractor Costs | | | | Other Costs | | | |
|-----------|----|--------------------|-------------|--------------|---------------------|-----------------------------|-----------------|-------------------|--------------|-------------|--------------|-------------|---------------------|-------------|--------------|
| | | Contingency | | | | | | Bonds + Insurance | | | Profit | | Sales Tax | Sub-Total | Design 8% |
| | | Direct Costs | 25% | Sub-Total | | | | Sub-Total | 6% | Sub-Total | 8.1% | Sub-Total | Total | | |
| 2022-2027 | 3 | \$1,451,000 | \$362,750 | \$1,813,750 | \$18,138 | \$145,100 | \$197,699 | \$43,494 | \$2,218,180 | \$133,091 | \$2,351,271 | \$190,453 | \$2,541,724 | \$203,338 | \$2,745,062 |
| | 4 | \$1,620,810 | \$405,203 | \$2,026,013 | \$20,260 | \$162,081 | \$220,835 | \$48,584 | \$2,477,773 | \$148,666 | \$2,626,439 | \$212,742 | \$2,839,181 | \$227,134 | \$3,066,315 |
| | 5 | \$897,000 | \$224,250 | \$1,121,250 | \$11,213 | \$89,700 | \$122,216 | \$26,888 | \$1,371,266 | \$82,276 | \$1,453,542 | \$117,737 | \$1,571,279 | \$125,702 | \$1,696,982 |
| | 6 | \$1,604,000 | \$401,000 | \$2,005,000 | \$20,050 | \$160,400 | \$218,545 | \$48,080 | \$2,452,075 | \$147,124 | \$2,599,199 | \$210,535 | \$2,809,735 | \$224,779 | \$3,034,513 |
| | 7 | \$1,782,000 | \$445,500 | \$2,227,500 | \$22,275 | \$178,200 | \$242,798 | \$53,415 | \$2,724,188 | \$163,451 | \$2,887,639 | \$233,899 | \$3,121,538 | \$249,723 | \$3,371,261 |
| | 8 | \$20,700,000 | \$5,175,000 | \$25,875,000 | \$258,750 | \$2,070,000 | \$2,820,375 | \$620,483 | \$31,644,608 | \$1,898,676 | \$33,543,284 | \$2,717,006 | \$36,260,290 | \$2,900,823 | \$39,161,113 |
| | | | | | | | | | | | | | TOTAL (2022 - 2027) | | \$53,075,246 |
| 2027-2032 | 9 | \$1,604,000 | \$401,000 | \$2,005,000 | \$20,050 | \$160,400 | \$218,545 | \$48,080 | \$2,452,075 | \$147,124 | \$2,599,199 | \$210,535 | \$2,809,735 | \$224,779 | \$3,034,513 |
| | 10 | \$3,071,000 | \$767,750 | \$3,838,750 | \$38,388 | \$307,100 | \$418,424 | \$92,053 | \$4,694,714 | \$281,683 | \$4,976,397 | \$403,088 | \$5,379,486 | \$430,359 | \$5,809,844 |
| | 11 | \$2,379,000 | \$594,750 | \$2,973,750 | \$29,738 | \$237,900 | \$324,139 | \$71,311 | \$3,636,837 | \$218,210 | \$3,855,047 | \$312,259 | \$4,167,306 | \$333,384 | \$4,500,890 |
| | | | | | | | | | | | | | TOTAL (2027 - 2032) | | \$13,345,048 |
| | | | | | | | | | | | | | TOTAL | | \$66,420,294 |

Gravity Pipe

Improvement ID: 3

Design Parameters:

| | |
|-------------|---------|
| Pipe Length | 3600 FT |
| Pipe Size | 24 IN |

Assumptions:

| Fiscal Year: | 22/23 | 23/24 | 24/25 | 25/26 | 26/27 | 27/28 | 28/29 | 29/30 | 30/31 | 31/32 | Total |
|--------------|-------|-------------|-------|-------|-------|-------|-------|-------|-------|-------|-------------|
| CIP Budget: | | \$1,451,000 | | | | | | | | | \$1,451,000 |

Construction Costs

| | |
|-----------------------------------|--------------------|
| Construction | \$1,451,000 |
| Contingency @ 25% | \$362,750 |
| Subtotal Construction Cost | \$1,813,750 |

Contractor Costs

| | |
|---------------------------------|------------------|
| Mob/Demob (1%) | \$18,138 |
| General Conditions (8%) | \$145,100 |
| Overhead (10%) | \$197,699 |
| Bonds + Insurance (2%) | \$43,494 |
| Profit (6%) | \$133,091 |
| Subtotal Contractor Cost | \$537,521 |

Other Costs

| | |
|-----------------------------|------------------|
| Sales Tax (8.1%) | \$190,453 |
| Engineering Design (8%) | \$203,338 |
| Subtotal Other Costs | \$393,791 |

Total Project Cost **\$2,745,062**

| Construction Costs | Unit | Quantity | Unit Cost | Extended Cost |
|-----------------------------------|------|----------|-----------|--------------------|
| Gravity Pipe | LF | 3600 | \$331 | \$1,192,320 |
| Manholes | EA | 9 | \$28,750 | \$258,750 |
| | | | | \$1,451,000 |
| Sub-Total | | | | \$362,750 |
| Contingency (25%) | | | | |
| Subtotal Construction Cost | | | | \$1,813,750 |

Gravity Pipe

Improvement ID: 4

Design Parameters:

| | |
|-------------|---------|
| Pipe Length | 5300 FT |
| Pipe Size | 27 IN |

Assumptions: 31% of construction costs are assigned to existing 15-inch line, and 69% of construction costs are assigned to the new 27-inch line

| Fiscal Year: | 22/23 | 23/24 | 24/25 | 25/26 | 26/27 | 27/28 | 28/29 | 29/30 | 30/31 | 31/32 | Total |
|--------------|-------|-------------|-------|-------|-------|-------|-------|-------|-------|-------|-------------|
| CIP Budget: | | \$2,349,000 | | | | | | | | | \$2,349,000 |

Construction Costs

| | |
|-----------------------------------|--|
| Construction | \$1,620,810 (assume 69% of total construction costs) |
| Contingency @ 25% | \$405,203 |
| Subtotal Construction Cost | \$2,026,013 |

Contractor Costs

| | |
|---------------------------------|------------------|
| Mob/Demob (1%) | \$20,260 |
| General Conditions (8%) | \$162,081 |
| Overhead (10%) | \$220,835 |
| Bonds + Insurance (2%) | \$48,584 |
| Profit (6%) | \$148,666 |
| Subtotal Contractor Cost | \$600,427 |

Other Costs

| | |
|-----------------------------|------------------|
| Sales Tax (8.1%) | \$212,742 |
| Engineering Design (8%) | \$227,134 |
| Subtotal Other Costs | \$439,876 |

Total Project Cost **\$3,066,315**

| Construction Costs | Unit | Quantity | Unit Cost | Extended Cost |
|--------------------|-----------------------------------|----------|-----------|--------------------|
| Gravity Pipe | LF | 5300 | \$373 | \$1,974,780 |
| Manholes | EA | 13 | \$28,750 | \$373,750 |
| | Sub-Total | | | \$2,349,000 |
| Contingency (25%) | | | | \$587,250 |
| | Subtotal Construction Cost | | | \$2,936,250 |

Gravity Pipe

Improvement ID: 5

Design Parameters:

| | |
|-------------|---------|
| Pipe Length | 2800 FT |
| Pipe Size | 27 IN |

Assumptions:

| Fiscal Year: | 22/23 | 23/24 | 24/25 | 25/26 | 26/27 | 27/28 | 28/29 | 29/30 | 30/31 | 31/32 | Total |
|--------------|-------|-----------|-------|-------|-------|-------|-------|-------|-------|-------|-----------|
| CIP Budget: | | \$897,000 | | | | | | | | | \$897,000 |

Construction Costs

| | |
|-----------------------------------|--------------------|
| Construction | \$897,000 |
| Contingency @ 25% | \$224,250 |
| Subtotal Construction Cost | \$1,121,250 |

Contractor Costs

| | |
|---------------------------------|------------------|
| Mob/Demob (1%) | \$11,213 |
| General Conditions (8%) | \$89,700 |
| Overhead (10%) | \$122,216 |
| Bonds + Insurance (2%) | \$26,888 |
| Profit (6%) | \$82,276 |
| Subtotal Contractor Cost | \$332,292 |

Other Costs

| | |
|-----------------------------|------------------|
| Sales Tax (8.1%) | \$117,737 |
| Engineering Design (8%) | \$125,702 |
| Subtotal Other Costs | \$243,439 |

Total Project Cost **\$1,696,982**

| Construction Costs | Unit | Quantity | Unit Cost | Extended Cost |
|-----------------------------------|------|----------|-----------|--------------------|
| Gravity Pipe | LF | 2800 | \$248 | \$695,520 |
| Manholes | EA | 7 | \$28,750 | \$201,250 |
| | | | | \$897,000 |
| Sub-Total | | | | \$224,250 |
| Contingency (25%) | | | | |
| Subtotal Construction Cost | | | | \$1,121,250 |

Gravity Pipe

Improvement ID: 6

Design Parameters:

| | |
|-------------|---------|
| Pipe Length | 5300 FT |
| Pipe Size | 27 IN |

Assumptions:

| Fiscal Year: | 22/23 | 23/24 | 24/25 | 25/26 | 26/27 | 27/28 | 28/29 | 29/30 | 30/31 | 31/32 | Total |
|--------------|-------|-------------|-------|-------|-------|-------|-------|-------|-------|-------|-------------|
| CIP Budget: | | \$1,604,000 | | | | | | | | | \$1,604,000 |

Construction Costs

| | |
|-----------------------------------|--------------------|
| Construction | \$1,604,000 |
| Contingency @ 25% | \$401,000 |
| Subtotal Construction Cost | \$2,005,000 |

Contractor Costs

| | |
|---------------------------------|------------------|
| Mob/Demob (1%) | \$20,050 |
| General Conditions (8%) | \$160,400 |
| Overhead (10%) | \$218,545 |
| Bonds + Insurance (2%) | \$48,080 |
| Profit (6%) | \$147,124 |
| Subtotal Contractor Cost | \$594,199 |

Other Costs

| | |
|-----------------------------|------------------|
| Sales Tax (8.1%) | \$210,535 |
| Engineering Design (8%) | \$224,779 |
| Subtotal Other Costs | \$435,314 |

Total Project Cost **\$3,034,513**

| Construction Costs | Unit | Quantity | Unit Cost | Extended Cost |
|-----------------------------------|------|----------|-----------|--------------------|
| Gravity Pipe | LF | 5300 | \$248 | \$1,316,520 |
| Manholes | EA | 10 | \$28,750 | \$287,500 |
| | | | | \$1,604,000 |
| Sub-Total | | | | |
| Contingency (25%) | | | | \$401,000 |
| Subtotal Construction Cost | | | | \$2,005,000 |

Gravity Pipe

Improvement ID: 7

Design Parameters:

| | |
|-------------|---------|
| Pipe Length | 5900 FT |
| Pipe Size | 27 IN |

Assumptions:

| Fiscal Year: | 22/23 | 23/24 | 24/25 | 25/26 | 26/27 | 27/28 | 28/29 | 29/30 | 30/31 | 31/32 | Total |
|--------------|-------|-------------|-------|-------|-------|-------|-------|-------|-------|-------|-------------|
| CIP Budget: | | \$1,782,000 | | | | | | | | | \$1,782,000 |

Construction Costs

| | |
|-----------------------------------|--------------------|
| Construction | \$1,782,000 |
| Contingency @ 25% | \$445,500 |
| Subtotal Construction Cost | \$2,227,500 |

Contractor Costs

| | |
|---------------------------------|------------------|
| Mob/Demob (1%) | \$22,275 |
| General Conditions (8%) | \$178,200 |
| Overhead (10%) | \$242,798 |
| Bonds + Insurance (2%) | \$53,415 |
| Profit (6%) | \$163,451 |
| Subtotal Contractor Cost | \$660,139 |

Other Costs

| | |
|-----------------------------|------------------|
| Sales Tax (8.1%) | \$233,899 |
| Engineering Design (8%) | \$249,723 |
| Subtotal Other Costs | \$483,622 |

Total Project Cost **\$3,371,261**

| Construction Costs | Unit | Quantity | Unit Cost | Extended Cost |
|-----------------------------------|------|----------|-----------|--------------------|
| Gravity Pipe | LF | 5900 | \$248 | \$1,465,560 |
| Manholes | EA | 11 | \$28,750 | \$316,250 |
| | | | | \$1,782,000 |
| Sub-Total | | | | \$445,500 |
| Contingency (25%) | | | | |
| Subtotal Construction Cost | | | | \$2,227,500 |

Marana WRF Expansion

Improvement ID: 8

Design Parameters:

Water Reclamation Facility 1.5 MGD

Assumptions:

| Fiscal Year: | 22/23 | 23/24 | 24/25 | 25/26 | 26/27 | 27/28 | 28/29 | 29/30 | 30/31 | 31/32 | Total |
|--------------|-------|-------|-------------|-------------|-------------|-------|-------|-------|-------|-------|--------------|
| CIP Budget: | | | \$2,900,823 | \$9,000,000 | \$9,000,000 | | | | | | \$20,900,823 |

Construction Costs

| | |
|-----------------------------------|---------------------|
| Construction | \$20,700,000 |
| Contingency @ 25% | \$5,175,000 |
| Subtotal Construction Cost | \$25,875,000 |

Contractor Costs

| | |
|---------------------------------|--------------------|
| Mob/Demob (1%) | \$258,750 |
| General Conditions (8%) | \$2,070,000 |
| Overhead (10%) | \$2,820,375 |
| Bonds + Insurance (2%) | \$620,483 |
| Profit (6%) | \$1,898,676 |
| Subtotal Contractor Cost | \$7,668,284 |

Other Costs

| | |
|-----------------------------|--------------------|
| Sales Tax (8.1%) | \$2,717,006 |
| Engineering Design (8%) | \$2,900,823 |
| Subtotal Other Costs | \$5,617,829 |

Total Project Cost **\$39,161,113**

| Construction Costs | Unit | Quantity | Unit Cost | Extended Cost |
|--------------------|------|----------|--------------|---------------------|
| Treatment | EA | 1.5 | \$13,800,000 | \$20,700,000 |
| | | | | \$20,700,000 |
| Contingency (25%) | | | | \$5,175,000 |
| | | | | \$25,875,000 |

Gravity Pipe

Improvement ID: 9

Design Parameters:

| | |
|-------------|---------|
| Pipe Length | 5300 FT |
| Pipe Size | 27 IN |

Assumptions:

| Fiscal Year: | 22/23 | 23/24 | 24/25 | 25/26 | 26/27 | 27/28 | 28/29 | 29/30 | 30/31 | 31/32 | Total |
|--------------|-------|-------|-------|-------|-------|-------|-------|-------|-------------|-------|-------------|
| CIP Budget: | | | | | | | | | \$1,604,000 | | \$1,604,000 |

Construction Costs

| | |
|-----------------------------------|--------------------|
| Construction | \$1,604,000 |
| Contingency @ 25% | \$401,000 |
| Subtotal Construction Cost | \$2,005,000 |

Contractor Costs

| | |
|---------------------------------|------------------|
| Mob/Demob (1%) | \$20,050 |
| General Conditions (8%) | \$160,400 |
| Overhead (10%) | \$218,545 |
| Bonds + Insurance (2%) | \$48,080 |
| Profit (6%) | \$147,124 |
| Subtotal Contractor Cost | \$594,199 |

Other Costs

| | |
|-----------------------------|------------------|
| Sales Tax (8.1%) | \$210,535 |
| Engineering Design (8%) | \$224,779 |
| Subtotal Other Costs | \$435,314 |

Total Project Cost **\$3,034,513**

| Construction Costs | Unit | Quantity | Unit Cost | Extended Cost |
|-----------------------------------|------|----------|-----------|--------------------|
| Gravity Pipe | LF | 5300 | \$248 | \$1,316,520 |
| Manholes | EA | 10 | \$28,750 | \$287,500 |
| | | | | \$1,604,000 |
| Sub-Total | | | | |
| Contingency (25%) | | | | \$401,000 |
| Subtotal Construction Cost | | | | \$2,005,000 |

Gravity Pipe

Improvement ID: 10

Design Parameters:

| | |
|-------------|----------|
| Pipe Length | 11500 FT |
| Pipe Size | 15 IN |

Assumptions:

| Fiscal Year: | 22/23 | 23/24 | 24/25 | 25/26 | 26/27 | 27/28 | 28/29 | 29/30 | 30/31 | 31/32 | Total |
|--------------|-------|-------|-------|-------|-------|-------|-------|-------|-------------|-------|-------------|
| CIP Budget: | | | | | | | | | \$3,071,000 | | \$3,071,000 |

Construction Costs

| | |
|-----------------------------------|--------------------|
| Construction | \$3,071,000 |
| Contingency @ 25% | \$767,750 |
| Subtotal Construction Cost | \$3,838,750 |

Contractor Costs

| | |
|---------------------------------|--------------------|
| Mob/Demob (1%) | \$38,388 |
| General Conditions (8%) | \$307,100 |
| Overhead (10%) | \$418,424 |
| Bonds + Insurance (2%) | \$92,053 |
| Profit (6%) | \$281,683 |
| Subtotal Contractor Cost | \$1,137,647 |

Other Costs

| | |
|-----------------------------|------------------|
| Sales Tax (8.1%) | \$403,088 |
| Engineering Design (8%) | \$430,359 |
| Subtotal Other Costs | \$833,447 |

Total Project Cost **\$5,809,844**

| Construction Costs | Unit | Quantity | Unit Cost | Extended Cost |
|-----------------------------------|------|----------|-----------|--------------------|
| Gravity Pipe | LF | 11500 | \$207 | \$2,380,500 |
| Manholes | EA | 24 | \$28,750 | \$690,000 |
| | | | | \$3,071,000 |
| Sub-Total | | | | \$767,750 |
| Contingency (25%) | | | | |
| Subtotal Construction Cost | | | | \$3,838,750 |

Gravity Pipe, Lift Station, and Force Main

Improvement ID: 11

Design Parameters:

| | |
|-----------------|------------|
| Pipe Length | 6800 FT |
| Pipe Size | 18 IN |
| Lift Station | 210 gpm |
| | 0.3024 MGD |
| Force Main | 6800 FT |
| Force Main Size | 6 IN |

Assumptions:

| Fiscal Year: | 22/23 | 23/24 | 24/25 | 25/26 | 26/27 | 27/28 | 28/29 | 29/30 | 30/31 | 31/32 | Total |
|--------------|-------|-------|-------|-------|-------|-------|-------|-------|-------------|-------|-------------|
| CIP Budget: | | | | | | | | | \$2,379,000 | | \$2,379,000 |

Construction Costs

| | |
|-----------------------------------|--------------------|
| Construction | \$2,379,000 |
| Contingency @ 25% | \$594,750 |
| Subtotal Construction Cost | \$2,973,750 |

Contractor Costs

| | |
|---------------------------------|------------------|
| Mob/Demob (1%) | \$29,738 |
| General Conditions (8%) | \$237,900 |
| Overhead (10%) | \$324,139 |
| Bonds + Insurance (2%) | \$71,311 |
| Profit (6%) | \$218,210 |
| Subtotal Contractor Cost | \$881,297 |

Other Costs

| | |
|-----------------------------|------------------|
| Sales Tax (8.1%) | \$312,259 |
| Engineering Design (8%) | \$333,384 |
| Subtotal Other Costs | \$645,643 |

Total Project Cost \$4,500,690

| Construction Costs | Unit | Quantity | Unit Cost | Extended Cost |
|-----------------------------------|------|----------|-----------|--------------------|
| Gravity Pipe | LF | 6800 | \$248 | \$1,689,120 |
| Manholes | EA | 24 | \$28,750 | \$690,000 |
| Lift Station | MGD | 0.302 | \$690,000 | \$208,656 |
| Force Main | LF | 6800 | \$55 | \$375,360 |
| Sub-Total | | | | \$2,379,000 |
| Contingency (25%) | | | | \$594,750 |
| Subtotal Construction Cost | | | | \$2,973,750 |